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Title

Effect of urbanization on stream hydraulics

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<https://escholarship.org/uc/item/0g43w23q>

Journal

River Research and Applications, 34(7)

ISSN

1535-1459

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Publication Date

2018-09-01

DOI

10.1002/rra.3293

Peer reviewed

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3 Running head: Urbanization and stream hydraulics

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14 **Cite as:** Anim, D.O., Fletcher, T.D., Vietz, G.J., Pasternack, G.B., Burns, M.J. 2018. Effect
15 of urbanization on stream hydraulics, *River Research Applications*, 1–14.

16 DOI: 10.1002/rra.3293

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25 **ABSTRACT**

26 Urbanization results in major changes to stream morphology and hydrology with the latter often
27 cited as a primary stressor of urban stream ecosystems. These modifications unequivocally
28 alter stream hydraulics, but little is known about such impacts. Hydraulic changes due to
29 urbanization were demonstrated using two-dimensional hydrodynamic model simulations,
30 comparing urban and non-urban stream reaches. We investigated three ecologically relevant
31 hydraulic characteristics; bed mobilization, retentive habitat and floodplain inundation, using
32 hydraulic metrics bed shear stress, shallow slow-water habitat (SSWH) area, and floodplain
33 inundation area. We hypothesized that urbanization would substantially increase bed
34 mobilization, decrease retentive habitat and due to increased channel size would decrease
35 floodplain inundation. Relative percent area of bed disturbance was four times higher,
36 compared with that of the non-urban stream at bankfull discharge (Q_{bkf}). SSWH availability
37 rapidly diminished in the urban stream as discharge increased, with SSWH area and patch size
38 two times smaller than the non-urban stream for a frequently occurring flow 0.7 times Q_{bkf} .
39 Floodplain inundation decreased in frequency and duration. These results demonstrate changes
40 in hydraulics due to urbanization that may impact on physical habitat in streams. New “water
41 sensitive” approaches to stormwater management could be enhanced by specification of
42 hydraulic regimes capable of supporting healthy stream habitats. We propose that a complete
43 management approach should include the goals of restoration and protection of natural
44 hydraulic processes, particularly those that support ecological and geomorphic functioning of
45 streams.

46

47 **Keywords:** urbanization; streams; hydraulic; stormwater runoff; channel morphology; flow
48 regimes; stream management

49 1. INTRODUCTION

50 Most streams draining urban catchments exhibit signs of ecological degradation (Morley &
51 Karr, 2002; Wenger et al., 2009; King et al., 2011). Recent studies point to urban stormwater
52 runoff as a primary degrader of stream ecosystems (Ladson et al., 2006; Burns et al., 2012;
53 Walsh et al., 2012; Vietz et al., 2014). When urban stormwater runoff (runoff from impervious
54 areas) is conveyed directly to streams via conventional stormwater drainage, many impacts
55 occur including increased frequency of hydrologic and water quality disturbance, as well as
56 channel geomorphology alteration (Brabec et al., 2002; Walsh et al., 2012; Vietz et al., 2015).
57 Combined, these impacts result in poor in-stream ecological condition, often referred to as the
58 'urban stream syndrome' (Walsh et al., 2005c). Whilst evaluating multiple stressors is an area
59 of active research (Meyer et al., 2005; Wenger et al., 2009), the frequent disturbance through
60 regular delivery of polluted stormwater runoff is considered a 'master variable' influencing
61 stream health (Walsh et al., 2012).

62 The altered flow regimes and consistent changes to stream ecosystems following urbanization
63 particularly stormwater management as an agent is now well recognised (Paul & Meyer, 2008;
64 Walsh et al., 2012). It is characterised by: 1) frequent flashy (with increased magnitude,
65 volume, steep rising and falling limbs) hydrograph as a result of impervious surfaces directly
66 connected to the streams; 2) increased frequency of flows below the long-term mean daily flow
67 rate; and 3) decreased summer and winter baseflow magnitude because of reduced infiltration
68 (Walsh et al., 2005b; Burns et al., 2012). Consequently, with the increased frequency,
69 magnitude and duration of altered flows, urban stormwater runoff is considered highly effective
70 geomorphic agent. Together with changes in sediment supply, urban streams in response to
71 altered hydrology experience widespread adjustments in the stream channel including
72 enlargement, deepening and simplification of channel morphology (Bledsoe & Watson, 2001;
73 Reinfelds et al., 2004; Hawley et al., 2012; Vietz et al., 2014). These changes are reported to
74 increase as the level of connected imperviousness increased (Hawley & Bledsoe, 2011). For
75 example, Vietz et al. (2014) found a correlation between connected imperviousness, and
76 geomorphic change in urban streams. They reported that urban streams even with less than
77 ~3% connected impervious area showed degraded channel with less variability, unless the
78 channels are subject to constraints such as bank or bedrock protection. Others have also
79 correlated hydrological changes driven by increases in impervious surfaces to changes channel
80 form and stability (see Chin, 2006; Vietz et al., 2016).

81 This has resulted in urban stream degradation being considered a predominantly hydrological
82 problem (Walsh, 2004; Roy et al., 2005; Burns et al., 2012), albeit other direct in-stream
83 physical intervention works such as channelization, channel straightening is also recognized to
84 cause channel degradation (Chin, 2006). This has resulted in driving research to understand the
85 mechanisms influencing degradation, and to inform protection and restoration approaches
86 (Wenger et al., 2009). Thus, substantial effort has been given to using hydrological-based
87 approaches for assessing instream flow regimes and understanding stream responses to guide
88 informed management decisions (Wenger et al., 2009; Burns et al., 2012). It has been
89 hypothesized that frequent flow disturbance is strongly linked to stream ecosystem
90 degradation, including morphological and ecological impairment (Walsh et al., 2005a; Vietz et
91 al., 2014). This suggests the need to address frequent flow input from particularly impervious
92 areas in the urban catchment.

93 While hydrology is a primary stressor, consideration of hydrology alone fails to recognise the
94 role of channel morphology in translating flow into hydraulic characteristics, such as depth and
95 velocity. The ecological relevant of hydraulic conditions produced by the interaction of
96 streamflow and in-channel physical features is widely recognised (Kemp et al., 2000; Turner
97 & Stewardson, 2014). Stream hydraulic conditions are known to drive ecosystem structure and
98 function (Statzner & Higler, 1986; Jowett, 2003; Brooks et al., 2005). The relationships
99 between reach-scale hydraulics metrics have been used as basis to inform environmental flow
100 management (Acreman & Dunbar, 2004; Turner & Stewardson, 2014) and also to quantify
101 ecologically important stream functioning (Steuer et al., 2009). For example, the duration and
102 area of habitat availability and refuge for biota provided within the wetted channel have
103 primarily been investigated which is mostly essential determinant of species population
104 dynamics (Gibbins et al., 2007; Lobera et al., 2017). Particularly, shallow slow-water habitats
105 (SSWH) are vulnerable to hydrological alteration and have been shown to reduce fish
106 abundance, macroinvertebrates that depend on SSWH as refugia and organic matter retention
107 (Vietz et al., 2013). Another example is the bed shear stress usually examined to address refuge
108 concept for benthic biota where duration of reach-average shear stress above specific threshold
109 are used to assess possible disturbance of biota from bed exposure (Jorde & Bratrich, 1998;
110 Mérioux & Dolédec, 2004). In addition, the frequency, duration and extent of floodplains
111 flows have been linked with flow-mediated exchange of energy, organic matter and biota
112 (Cienciala & Pasternack, 2017). Floodplains flows dynamics produce important habitat
113 supporting biota such as fish utilizing it as spawning and rearing habitat (Gorski et al., 2011).

114 In most aquatic ecosystem flow investigation, the spatial and temporal variabilities of these
115 hydraulic conditions have been closely linked to the ecological condition (Humphries et al.,
116 2006; Vietz et al., 2013) as well as geomorphic processes (Strom et al., 2016). Thus, directly
117 linking hydrologic indicators to stream ecosystem impairment without considering hydraulics,
118 fails to account for the direct causal physical mechanisms driving stream degradation and
119 habitat quality (Escobar-Arias & Pasternack, 2010).

120 Recent studies have argued that environmental flow evaluations must go beyond just
121 hydrologic assessment and include hydrogeomorphic processes that are directly linked to the
122 needs of the aquatic ecosystem (e.g., Wohl et al., 2015; Yarnell et al., 2015). In the attempts to
123 understand the mechanistic pathways of urban-induced changes leading to stream degradation
124 to inform management, studies have rarely considered the hydraulic responses beyond a
125 general understanding of an increase in stream power. It is often the hydraulic conditions that
126 influence biota and ecosystem functioning. That the relationship between hydraulics and
127 instream form and function are often used to speculate the mechanisms influencing ecological
128 structure and functions points to the importance of exploring these relationships. However,
129 there is limited understanding of how the hydraulic conditions, particularly those relevant to
130 ecosystem health, are influenced by the compounded urban-induced hydrological alterations
131 alongside morphology change. The understanding of the altered hydraulic environment in
132 urban streams currently provides a poor foundation for protection or restoration, i.e. the relative
133 role of addressing hydrology or channel morphology. Since the hydraulic conditions are poorly
134 quantified they are rarely a focus for management, leading to suggestions that this may be a
135 reason for the lack of desired ecological improvements (Clark et al., 2008; Violin et al., 2011).

136 In this study, we aimed to evaluate hydraulic changes in an urban stream as a result of altered
137 catchment hydrology and channel morphology. To investigate this, two-dimensional (2D)
138 hydrodynamic modelling was used to characterize and compare hydraulics in urban and non-
139 urban reaches of the same stream. We characterized the degree of hydraulic change using three
140 ecologically relevant metrics that describe (i) the extent of the channel bed disturbance, (ii) the
141 hydraulic habitat availability (using SSWH); and (iii) floodplain inundation (which drives
142 hydrologic connectivity between stream channels and floodplains). These hydraulic metrics are
143 important indicators for aquatic ecosystem and biotic functioning (McCabe & Gotelli, 2000;
144 Paterson & Whitfield, 2000; King et al., 2003; Brooks et al., 2005). Our study aims to underpin
145 a better mechanistic understanding of the relationships between urban-induced stormwater

146 runoff and degradation of stream ecosystems and thus help improve the outcomes of stream
147 restoration and protection activities.

148 **2. METHODS**

149 Field data collection was performed to characterize the fluvial terrain and hydrology of two
150 stream reaches to enable mechanistic, 2D modeling over a range of discharges. Hydraulic
151 calibration and validation data were also collected. Two-dimensional hydraulic models were
152 produced using TUFLOW 2D. Outputs of bed shear stress, velocity and depth were obtained
153 from steady flow simulations. Results were analyzed to assess bed disturbance pattern,
154 hydraulic habitat availability, and floodplain inundation extent. Further details of the study
155 sites and field data collection are provided in Supplementary Materials.

156 *2.1. Study sites*

157 The study was carried out on Cardinia Creek, which flows 34km south to Western Port Bay in
158 south-eastern Melbourne, Australia (Figure 1). Two study reaches were selected to physically
159 represent and compare non-urban and urban settings, referred to herein as the ‘non-urban site’
160 and ‘urban site’ respectively (Table 1). The non-urban site, located 6 km upstream of the urban
161 site is a comparatively intact and complex naturally meandering channel with a sand-gravel
162 bed, well-defined riffle-pool, benches and point bar morphological features. The urban site has
163 a relatively simplified low-gradient, sand-gravel bed channel morphology, and exhibits less
164 complexity both in planform and cross-profile. Estimated average bankfull width dimensions
165 from LiDAR along the Cardinia creek segments draining non-urban portions of the Cardinia
166 Shire catchment to the segments draining the increasingly urbanized downstream portions
167 indicated a progressive change in channel dimensions and planform as the stream move
168 towards the urban areas (see figure in Supplementary materials as Figure S1). This is typical
169 of urbanized streams which tends to have wider channels (through incision and bank erosion
170 from increased runoff) (Walsh, 2004; Hawley & Bledsoe, 2011). While the urban site channel
171 was selected to represent a channel predominantly impacted by urban-induced hydrological
172 changes, a section of the reach flows under a bridge which potentially could have somehow
173 influenced the current channel morphology. This site represents a channel typical of those
174 draining urban catchments in this region.

175

176

177 *2.2. Data collection*

178 Detailed topographic surveys and hydrology data were collected on each study reach to enable
179 2D modelling. Surveying covered channel and floodplain areas. It was used to derive a DEM
180 to elevate the computational mesh for each reach. Hydrologic data provided streamflow
181 statistics and enabled the selection of the range of flows to be modelled. Hydraulic data were
182 also sampled for model calibration and validation.

183 *2.2.1. Channel topography*

184 At each site, a 100-m study reach was selected corresponding to about 20 times bankfull
185 channel width. Topographic and bathymetric data were gathered using a Sokkia Set 5X total
186 station and Leica Viva GS15 GNSS receiver. Survey data described the channel bed and banks,
187 water surface elevation (WSE), wet/dry edge boundaries. The channel bed was surveyed with
188 a lateral and longitudinal frequency of approximately 0.5 m for both sites. The particle size
189 distribution of bed materials was determined by pebble counts (Wolman, 1954) wherein the b-
190 axis of a minimum of 100 particles was measured. A representative median size (D_{50}) was
191 extracted from the particle size distribution for each site.

192 *2.2.2. Hydrology*

193 Water levels were monitored at the two study sites for one year using capacitive water level
194 sensors (ODYSSEY® MP System). The water level data were converted to discharge by means
195 of stage-discharge rating curves specifically estimated for the two study reaches based on direct
196 gauging (Figure 2). The sampling period provides a good representation of a typical
197 hydrologically average year in the catchment. For each discharge gauging, WSE longitudinal
198 profiling was done at 20 m intervals along both banks for each site. Further hydrologic detail
199 is provided in Supplementary Materials.

200 *2.3. Hydraulic modelling*

201 Hydraulic simulations were undertaken with the TUFLOW 2D model that solves the full two-
202 dimensional, depth-averaged momentum and continuity equations for free surface flow (Syme,
203 2001). A computational mesh was built with the bathymetric survey data for each site with
204 ~0.3 m grid size. The computational domain was extended about 20 m in both upstream and
205 downstream directions to reduce the impact of flow and boundary assumptions on model results
206 in the priority region of interest. Model input and boundary conditions for simulation runs were
207 inflow discharge and corresponding measured downstream WSE. The model was run in a

208 steady-state mode based on representative flows observed for each site during the study period
209 for discharges ranging from 0.04 to 3.35 m³/s, corresponding to 2-99 % of time discharge (Q)
210 exceedance. This range of simulated discharges represents 0.05-4 times and 0.02-2 times
211 bankfull discharge (Q_{bkf}) for the non-urban and urban site respectively.

212 2.4. Model calibration and validation

213 Model calibration was achieved by manipulation of the Manning's n values to match observed
214 WSE profiles. Model simulations were validated for flows ranging from 0.1-0.5 and 0.1-0.3
215 times Q_{bkf} for non-urban and urban sites, respectively, using measured fixed-point depth and
216 velocity sampled by wading. This was achieved by quantitatively comparing observed versus
217 modelled values in the direction of flow. Calibration and validation approach and metrics as
218 well as their threshold values are detailed in the Supplementary Material.

219 2.5. Habitat mapping and bed shear analysis

220 Bed shear stress outputs from the 2D model simulations were analyzed to compare the two
221 sites for their relative potential for bed particle entrainment at given flows. The non-
222 dimensionalized Shields Stress (τ^*) was used as a quantitative metric of the stability of the
223 channel bed (Pasternack, 2011), estimated from TUFLOW's bed shear stress results in each
224 grid cell as:

$$225 \tau^* = \frac{\tau_o}{D(\gamma_s - \gamma_w)} \quad (1)$$

226 where τ_o is the bed shear stress computed by TUFLOW, D is the representative particle size
227 of the channel bed (taken as D_{50} in this study), γ_s is the unit weight of bed particle and γ_w is the
228 unit weight of water. Shields stress values were then classified based on bed particle mobility
229 thresholds defined by Lisle et al. (2000), where $\tau^* < 0.03$ indicates stable bed or no mobility
230 and τ^* between 0.03 and 0.06 indicates intermittent entrainment, and $\tau^* > 0.06$ indicate likely
231 bed particle entrainment (e.g., Buffington & Montgomery, 1997; Escobar-Arias & Pasternack,
232 2010). Critical bed shear stress (τ_o^*) and τ^* was estimated using a single grain size (D_{50} = 6mm
233 for the urban site) for both sites.

234 For SSWH mapping and assessment, ArcGIS (Esri ArcGIS desktop 10.2) was used to process
235 and examine the depth and depth-average velocity outputs generated by the model simulations.
236 The outputs for the modeled 100-m domain at each simulated discharge were analyzed and
237 composite grid maps of velocity-depth outputs generated. The SSWH areas were mapped by

238 categorizing the grid cells that fell within a depth class of 0-0.3 m and velocity class of 0-0.2
239 ms^{-1} . While different combinations of depth and velocity classes have been shown to be
240 important to instream hydraulic habitat requirement for some species or some life stages, the
241 SSWH depth and velocity class considered here is reported to be preferred, particularly by
242 benthic macroinvertebrates in small streams (Shearer et al., 2015) and fish (Milhous & Nestler,
243 2016). The SSWH sub-metrics included the total SSWH patch area, mean SSWH patch size,
244 and SSWH patch density (number of SSWH patches divided by the channel length) (McGarigal
245 & Marks, 1995).

246 The area of delineation for the floodplain inundation analysis was limited to a buffer of 10 m
247 of the floodplain surface on each side of the stream channel. This is necessitated by the focus
248 on channel changes due to hydrologic change (i.e. larger capacity urban channel) rather than
249 imposed management changes (i.e. earthworks to restrict the floodplain). The approach used
250 here was to analyzed inundation extent and frequency in relation to discharge associated with
251 the 0.3%, 2%, 5%, 10% and 15% of time discharge exceeded. Although the 2D simulations
252 and hydraulic assessment were undertaken for different Q , for brevity in reporting the results,
253 the maps for the metrics (SSWH, Shields Stress) for the two sites were evaluated for baseflow,
254 median, Q_{bkf} and $2Q_{\text{bkf}}$ discharges.

255 *2.6. Data analysis*

256 The impacts of urbanization were assessed by looking at the increase or decrease of metrics as
257 a function of discharge relative to non-urban conditions. The magnitude of these changes for
258 each flow's wetted area was examined corresponding to a threshold value. For instance, the
259 degree of bed disturbance was examined in relation to thresholds for bed material entrainment
260 such as 0.03 or 0.06 for Shields stress. 2D maps of Shields stress, SSWH and floodplain
261 inundation were generated to assess patch behaviour and evaluate the extent of any longitudinal
262 changes. Changes as a function of discharge can be expansion, contraction, shifting and
263 emergence from non-existence (Brown et al., 2016).

264

265 **3. RESULTS**

266 *3.1. Model performance*

267 Comparing observed fixed-point velocity and depth data versus model predicted conditions
268 demonstrate satisfactory 2D model performance, with the points generally falling along a 1:1
269 line. See results, including figure (Figure S1) in Supplementary Materials.

270 3.2. Benthic disturbance

271 3.2.1. Bed shear stress patterns

272 The two study sites displayed different τ_o patterns, owing to their reach-scale morphological
273 differences. Both sites exhibit increased values of reach-averaged and maximum τ_o as Q
274 increased. At very low Q the rates appear similar, but then differ substantially at the urban site
275 getting close to Q_{bkf} , where for non-urban stream values stay relatively flat or increase
276 marginally (Figure 3).

277 The non-urban site showed the most stable bed with a reach-average τ^* of 0.02 at Q_{bkf} , below
278 the critical range of entrainment (~ 0.04) compared to 0.09 for the urban site. There was a sharp
279 increase in the portion of the wetted benthic area that is likely to have particles in full motion
280 at the urban site as Q increases, approximately 0.3-0.7x Q_{bkf} , representing flows exceeded
281 between 5-25% of the time (Figure 4). In contrast, the relative percent of the wetted bed area
282 potentially moving in the non-urban site remained small with increasing Q (6% at Q_{bkf}) and
283 begins to increase steadily for Q around 1.5x Q_{bkf} . This means that a greater portion of the non-
284 urban site channel bed retained low bed shear stress even as Q increased.

285 The frequency and magnitude of bed disturbance over the study period were predicted to be
286 substantially greater in the urban site than non-urban site (Figure 5). The period that the daily
287 maximum τ_o was equal to or exceeded the estimated τ_o^* (4 N/m²) was 120 days/year in the urban
288 site compared to 35 days/year in the non-urban site. For these periods, the maximum τ_o at the
289 urban site increases by a factor of 2-4. The estimated mean annual maximum τ_o was 2.79 N/m²
290 and 5.67 N/m² and the annual mean τ_o was 0.78 N/m² and 1.75 N/m² for the non-urban and
291 urban site respectively.

292 3.2.2. The spatial distribution of bed disturbance

293 Figure 6 shows planform maps of τ^* patch pattern for each site and across four discharges
294 representing baseflow, median discharge, Q_{bkf} and $2Q_{bkf}$ respectively. Each site had spatially
295 discrete regions of high bed disturbance and different patterns in how τ^* changes with Q .
296 Coherent areas of both decreases and increases in τ^* were observed as flow increases. Patches
297 of τ^* showed spatial patterns of shift, expansion, and contraction with increasing discharge.

298 These changing patterns were variable in both lateral and longitudinal dimensions, showing
299 diverse patch sizes and shapes. The τ^* was substantially higher in the urban channel compared
300 to the non-urban channel at high Q .

301 In the non-urban channel, greater variations in channel width and bed geometry mediated
302 where areas of high τ^* were observed to shift as Q increased. At $Q < Q_{\text{bkf}}$, areas of high τ^* in
303 the channel were mostly found at meander bends and topographic highs in the main channel.
304 Low terrain relief of the adjacent banks at the bends alleviated a fast expansion in the area of
305 high τ^* as discharge increased. High τ^* locations showed large lateral expansions as Q
306 increases within the bankfull channel. Areas high τ^* exhibited longitudinal extension, but
307 magnitude in these areas were mediated because of divergent flow and lateral expansion. While
308 these topographic features constricted the flow at $Q < Q_{\text{bkf}}$, they allowed rapid extension of the
309 effective flow area, dissipating the high τ^* that would have been expected as Q increases. The
310 shifts from lateral flow convergence to divergence change the core of high velocities from the
311 channel centre as flow increases thereby dissipating the energy and decreasing hydraulic forces
312 of the flow acting in the channel.

313 The lack of variation in channel width and bed geometry in the urban channel resulted in
314 reduced variation in the spatial location of high τ^* as flow increased, with areas simply
315 extending longitudinally and laterally. The relatively incised channel (compared to the non-
316 urban site) with steep bank constricted the flow in the channel as Q increased maintaining high
317 τ^* .

318 3.3. SSWH availability

319 3.3.1. SSWH changes with discharges

320 The total SSWH patch areas were different for the two study sites, given their morphological
321 differences, but the changes with Q showed a similar trend. In general, SSWH patch area was
322 high at low Q (Figure 7a) at $\sim 0.1-0.2 \times Q_{\text{bkf}}$ at both sites. At the urban site, as the predominantly
323 plane bed was inundated to greater depths at higher Q , velocity increased and a general decline
324 in the SSWH area was observed. However, following a brief decline in SSWH below Q_{bkf} the
325 SSWH area in the non-urban site steadily increased as Q approaches Q_{bkf} and rapidly increased
326 as the floodplain was inundated for $Q > Q_{\text{bkf}}$. This is supported by the gradually changing
327 topographic relief extending from the thalweg to the floodplain. Planform complexity allows
328 the inundation of new areas of lateral bars and benches, thus creating more SSWH. At low Q ,

329 the maximum SSWH patch area varied from 55 to 84 m²/100 m and 96 to 104 m²/100 m of the
330 wetted area for the non-urban site and the urban site respectively.

331 The mean SSWH patch size follows a similar pattern as the total area of SSWH (Figure 7b).
332 The SSWH patch size decreased (~2 times) rapidly at the urban site with increasing Q ,
333 particularly as Q approached Q_{bkf} compared to the non-urban site. Patch density consistently
334 increased with Q in the non-urban channel compared to the urban channel, where it decreased
335 as Q approached Q_{bkf} (Figure 7c).

336 Within-year availability of the total SSWH patch area during the study period (Figure 8)
337 revealed a considerable decline of SSWH availability in the urban site at high Q during the
338 winter period (June to September). In this period, the mean total area of SSWH patch was
339 ~35% greater in the non-urban site than the urban site. In contrast, the summer low flow periods
340 (December to March) showed a higher SSWH patch area in the urban site. During the periods
341 of $Q > Q_{bkf}$, the percentage of the floodplain area acting as SSWH in the non-urban site was 2-
342 4 times higher than the urban site.

343 3.3.2. *The spatial distribution of SSWH*

344 The SSWH occurred predominantly along the channel margins, expanding into the main
345 channel in both study sites at low flows (Figure 9). The patches shifted further to the channel
346 margins with increasing Q , but remained in large cohesive-linear patches in the non-urban
347 channel compared to the contracted and fragmented patches in the urban. SSWH patches in the
348 urban channel became more fragmented at higher Q compared to those at the non-urban site.

349 The broad, low topographic relief of the adjacent banks at the non-urban site facilitated a
350 greater spatial increase in SSWH patch area. This channel geometry allows more surface to be
351 inundated with shallow depths by lateral overflow with increase in Q . With $Q > Q_{bkf}$, there is
352 an increase in the spatial extent at which the floodplain is inundated at the non-urban site
353 compared to the urban site thus increasing the availability of the SSWH areas. The locations
354 of SSWH patches in the urban channel generally persisted, but patch area decreased with
355 increasing Q . On the contrary, SSWH locations in the non-urban channel migrated and
356 expanded or contracted with Q , reflecting the topographic dynamism of the stream channel at
357 this site.

358

359

360 *3.4. Floodplain inundation*

361 For the same flow exceedances, much less of the urban floodplain is inundated at the urban site
362 (Figure 10, 11). At Q_{bkf} , only 1% of the urban site floodplain area was inundated compared to
363 the 6% at the non-urban site. At $Q > Q_{bkf}$, much of the non-urban site floodplain was inundated
364 compared to the urban-site. At these Q , the area extent of floodplain inundation was ~5 times
365 larger than at the urban site.

366 Portions of the non-urban channel banks were overtopped for flows corresponding to 10 -15 %
367 of time Q exceeded ($Q < Q_{bkf}$). This appeared to be at low relief lateral portions of the non-
368 urban channel. The estimated frequency (days/year) of urban floodplain inundation over the
369 study period was estimated to be ~45% lower than the non-urban site. Furthermore, predicted
370 inundation duration was ~3 times higher at the non-urban site due to the longer cumulative
371 duration of peak events ($Q > Q_{bkf}$) compared to the flashiness at the urban site.

372 **4. DISCUSSION**

373 *4.1. Urbanization impacts on stream hydraulic conditions.*

374 Despite the geographic proximity of both sites investigated in this study, the influence of urban
375 stormwater inputs between the sites fundamentally alters hydraulic conditions. In this section
376 we discuss the three main findings from this study, and highlight the opportunities for better
377 understanding hydraulic alteration to improve the management of streams impacted by excess
378 urban stormwater runoff.

379 *4.1.1. Influence on benthic disturbance*

380 Local variations in bed shear stress acting on benthos influence sediment entrainment and
381 transport, which in turn drive the evolution of channel morphology. Changes in local flow
382 dynamics govern bed mobility from zones of higher to lower bed mobility (Lisle et al., 2000;
383 MacWilliams et al., 2006). The results indicate that the non-urban site would likely experience
384 substantially lower bed shear stress. In contrast, areas of the streambed retaining low bed shear
385 stress are limited in the urban site across the range of simulated discharges, consistent with the
386 view that benthic area available as refugia is rapidly diminished in urban or modified aquatic
387 systems whenever a flow event or spate occurs (Negishi et al., 2002; Finstad et al., 2007). The
388 comparatively confined, straight and relatively uniform gradient and cross-sectional profile at
389 the urban site account for the resultant rise in areas of potential bed entrainment as Q increases,
390 and thus loss of flow refugia.

391 It is widely recognised that the impact of altered flow on urban stream channel form eliminates
392 important morphological features (such as meanders, bars and benches, riffle-pool sequences)
393 (Chin, 2006; Vietz et al., 2014), thus decreasing channel variability. As shown in the non-urban
394 site (Figure 6), at the stream-reach scale, channel morphological heterogeneity steers flow in
395 such a way that the different topographic features turn on and off to create diverse patterns of
396 hydraulic conditions as Q increases (Strom et al., 2016). This suggests that morphological
397 heterogeneity will decrease areas of streambed that are subjected to high hydraulic stress with
398 rising flows. Consequently, benthic species assemblages in natural hydraulically complex
399 stream reaches are more persistent than in simple, modified ones (Negishi et al., 2002; Vericat
400 et al., 2008).

401 The area of channel experiencing likely bed entrainment rises rapidly for Q between 0.3 – 0.9x
402 Q_{bkf} (Figure 4), suggesting that management efforts to reduce bed disturbance should target
403 these flows for control by flow-regime restoration practices.

404 In this study, the frequency and duration of likely bed particle entrainment at the urban site was
405 substantially higher than for the non-urban site. The estimated daily peak shear stress equalled
406 or exceeded the critical shear stress for 120 days/year for the urban site, compared to 35
407 days/year in the non-urban site. This coincides with the hydrological observation of Wong et
408 al. (2000), who report that urban streams in Melbourne are typically disturbed by impervious
409 runoff more than 100 times/year. Local patch-scale benthic disturbance occurs even for
410 relatively small changes to Q . Vericat et al. (2008) reported that patches of sand-gravel bed
411 may attain partial or full entrainment even during smaller but more frequent flow events.

412 The geomorphic and thus ecological consequence of the modeled bed shear stress regime is
413 expected to be large, given that it will cause frequent entrainment of surface sediments and
414 eventually, mobilize subsurface particles. This activity can regularly adjust the physical habitat
415 (Francoeur & Biggs, 2006). A longer period of high bed shear stress combined with a lack of
416 peripheral SSWH will reduce the chance of benthic invertebrates finding refugia (Lancaster et
417 al., 2006; Oldmeadow et al., 2010). Removal of bed sediments is also the precursor to channel
418 incision (Hawley et al., 2012). This is consistent with studies hypothesising that streams in
419 urban catchments having a percentage connected impervious surface above 1% experience bed
420 movement, major incision and loss of sensitive biota resulting in decreased ecological quality
421 (Walsh et al., 2005a; Vietz et al., 2014). However, the magnitude of this phenomenon could
422 also depend on the sediment supply (Chin, 2006).

423 The findings here also suggest that considerable hydraulic alterations are expected even at the
424 low level of connected imperviousness (3% at the urban site), confirming the dominant role of
425 excess urban stormwater runoff in influencing hydraulic alteration. Vietz et al. (2014) similarly
426 reported large geomorphic changes in urban streams at very low levels of connected
427 imperviousness (<2-3%).

428 *4.1.2. Impacts on shallow slow-water habitat*

429 The channel geometry in the non-urban site ensures a gentler increase in depth laterally and
430 longitudinally, as Q increases. The channel wetted area increases without significant increases
431 in flow depth and velocity particularly towards channel margins. Thus, as Q increase, more
432 surface area is inundated with shallow depth and low velocity, increasing the SSWH area.
433 Conversely, the predominantly straight, uniform, plane-bed, U-shaped channel at the urban site
434 means there is less variability in flow depth and a steeper increase in depth and velocity with
435 increasing Q . Thus, as Q increases, the SSWH area decreases.

436 The SSWH and Q relationship observed for the urban and non-urban site can be compared to
437 the conceptual model defined by Vietz et al. (2013). For complex channels (with higher bars
438 and extensive shoals), as observed at the non-urban site in this study, the SSWH areas increase
439 as the high-level bars and extended shoals are inundated. Nevertheless, the rate of increase may
440 fall depending on the flow velocities over these features with increased flow depth (Knighton,
441 1974; Stewardson, 2005). For modified or simple channels (with near-vertical banks), as
442 observed at the urban site, rapid declines of SSWH area is expected, even at comparatively low
443 Q .

444 For the urban site, rapid declines in patch size (~2 times of SSWH patch size) were observed
445 for relatively small increases in Q compared to the non-urban site. This is expected to impact
446 species assemblages as the smaller the individual SSWH patches, the less chance species have
447 to survive progressive downstream drift (Vietz et al., 2013). In addition, the SSWH patches in
448 the urban site become more fragmented as Q increases. Reducing contiguousness of SSWH
449 patches lessens their ecological value (Dodd, 1990; Collinge, 1996), thus impacting ecological
450 diversity (Collinge, 1996; Ewers & Didham, 2006).

451 SSWH patches locations in the urban site were comparatively static, occurring at discrete zones
452 even as Q increased. In contrast, morphological heterogeneity at the non-urban channel allowed
453 large SSWH patches to be separated and distributed into many small units. Such diminishing

454 spatial heterogeneity in the urban site could contribute to species segregation and declines in
455 abundance and diversity (Collinge, 1996).

456 Habitat availability in the urban site clearly suggest that the modified channel together with
457 altered flow regime driven by urban impacts may provide limited SSWH habitat. In urban
458 catchments where streams experience increased frequency of peak flows (Burns et al., 2012;
459 Walsh et al., 2012), the decreased availability of SSWH can persist for long periods, reducing
460 rearing and breeding habitat and refuge. This could be a key contributing factor for local
461 extinction and declined diversity and abundance of biota (Diamond & Serveiss, 2001;
462 Poznańska et al., 2009; Wenger et al., 2009; Koperski, 2010). Aquatic systems with an
463 abundance of available SSWH are usually able to more effectively support diverse aquatic life
464 populations (West & Jones, 2001; Poznańska et al., 2009).

465 *4.1.3. Impacts on floodplain inundation*

466 The assessment of the inundation extent suggests a substantial impact of urbanization on the
467 floodplain inundation. The results on the estimated relative differences in the floodplain
468 inundation area at both sites reveals two general points. First, compared to the non-urban site,
469 our analysis shows that the frequency of floodplain inundation in the urban site is likely to
470 decrease. While altered catchment hydrology increases the magnitude and frequency of higher
471 discharge events (Figure 2), the increased channel capacity at the urban site would require a
472 very high, non-frequently occurring discharge to overtop the banks. This means the reach will
473 experience low rates of increase in inundation per unit flow, and consequently, a high reduction
474 in the inundated floodplain area. In addition, the duration of inundation is expected to be
475 reduced compared to non-urban analogues given the flashiness of high flows (Walsh et al.,
476 2012). Likewise, typically, confined incised stream reaches have limited floodplain space often
477 restricted by valley walls (e.g., Grant & Swanson, 1995; Vietz et al., 2015) as the case of the
478 urban site. While this geomorphic control limits the extent to which inundation can occur, it
479 also reduces the duration of the inundation (Cienciala & Pasternack, 2017).

480 We hypothesize that for urban streams with major changes to the flow regime and channel
481 form, the expected changes to the pattern (frequency and duration) of inundation will mean
482 altered lateral hydrologic connections between the stream and its floodplain. This could alter
483 seasonal timing and variability in the inundation pulse, potentially affecting the ability of
484 floodplain biota to cope with and gain from inundation (Kingsford, 2000; Hamilton et al.,
485 2002).

486 *4.2. Implications of hydraulics for ecosystem processes and restoration strategies in urban*
487 *streams*

488 The protection or restoration of urban streams requires understanding of the relationship
489 between catchment urbanization (particularly stormwater impacts) and a stream's physical and
490 biological process responses (Wenger et al., 2009). Figure 12 depicts a conceptual framework
491 of how individual stressors interact to impact the stream ecosystem. Hydraulic conditions are
492 the mediator between exogenous drivers (such as hydrology and morphology) and ecological
493 responses.

494 Until recently, altered channel morphology (Chin & Gregory, 2009; Vietz et al., 2016) and
495 hydrology (Wenger et al., 2009; Walsh et al., 2012) have been considered as the major
496 determinants of observed changes in stream physical and biological structure and function.
497 Management strategies to protect or restore urban streams typically involve either enhancing
498 the channel morphology, creating specific habitat characteristics to achieve perceived "better"
499 habitat conditions (Bernhardt & Palmer, 2011), or catchment-scale practices that aim to restore
500 flow regimes towards their pre-development levels. However, achieving ecologically
501 successful restoration still remains a struggle, in particular because morphological adjustments
502 usually do not address the underlying mechanisms of disturbance (Bernhardt & Palmer, 2011;
503 Violin et al., 2011). While flow-regime restoration efforts are more likely to do so, returning
504 to near natural levels can be very difficult (Duncan et al., 2014; Fletcher et al., 2014).

505 As demonstrated in this study, urban-induced altered hydrology and morphology have
506 substantial impacts on the stream hydraulic conditions. This potentially becomes a key agent
507 of declined ecological health usually observed in urban streams including declined diversity
508 and abundance of biota (Wenger et al., 2009). This means management efforts towards
509 protection and restoration of urban streams should incorporate objectives of achieving
510 ecologically relevant hydraulic conditions that would sustain the stream ecosystem health.
511 Indeed, it is well established that stream communities and many ecosystem functions depend
512 on hydraulic behaviour (Statzner et al., 1988; Gibbins et al., 2007; Clark et al., 2008; Knight
513 & Cuffney, 2012). Our findings suggest that stream hydraulic condition metrics can be assessed
514 by stream managers in their efforts to understand the mechanisms driving urban stream
515 degradation. They may be used to simulate and evaluate geomorphically and ecologically
516 important flow patterns within urban streams, to guide targeted restoration efforts both at the
517 catchment scale and within the channel (Pasternack & Brown, 2013; Brown et al., 2016).

518 Ultimately, it is necessary to set objectives that are directly linked to the needs of the receiving
519 stream. Given the need that managed flows and channel morphology should result in
520 sustainable geomorphic functioning (i.e. appropriate levels of erosion and deposition), and
521 suitable hydraulic habitat conditions for biotic functioning, building management standards
522 based on the hydraulic outcomes is a prerequisite to protecting the channel and restoring stream
523 ecosystems (Pasternack, 2008).

524 **5. CONCLUSIONS**

525 Altered stream hydrology driven by urban stormwater runoff is a key stressor to urban stream
526 ecosystems. In this study, we investigated and demonstrated the hydraulic response to flow in
527 an urban and non-urban stream, using 2D model simulations and three metrics that addressed
528 bed disturbance, SSWH, and floodplain inundation. The urban stream was found to have a
529 substantially altered hydraulic regime. Bed disturbance was nearly always greater in the urban
530 channel and approximately four times higher as flow increased, while SSWH availability in
531 the urban stream was greatly diminished. The areal extent of floodplain inundation in the urban
532 stream was limited to flood flows (i.e. rare events), indicating likely extended periods of lateral
533 disconnections between the stream and its floodplain.

534 The results highlight the important interplay between hydrology, geomorphology, and
535 hydraulics in dynamically evolving the discharge-hydraulic conditions in stream channels. In
536 urban streams considering either just hydrology, or just channel morphology, in isolation, may
537 not adequately achieve ecologically successful restoration. Restoration efforts should include
538 technical objectives to restore a natural hydraulic regime as part of a multi-scalar approach that
539 considers local, segment, and catchment scale concerns. Implementing such hydraulic-based
540 approaches will be helpful in prioritizing and integrating management efforts between
541 mitigation of hydrology and channel morphology interventions to achieve restoration targets.

542 **ACKNOWLEDGMENTS**

543 This work was funded by University of Melbourne Research Scholarship and the Melbourne
544 Waterway Research Practice Partnership, supported by Melbourne Water. We thank the
545 following staff of the Waterway Ecosystem Research Group of University of Melbourne who
546 assisted with field work: Peter Poelsma, Robert James, Michael Sammonds, Mathieu Bachaud
547 and Andrew Thomas. This paper benefited greatly from comments by Jason Wiener. T.
548 Fletcher was supported by Australian Research Council project FT100100144 during part of
549 this work.

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771 opportunities. *BioScience*, 65(10), 963-972.

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773

774 Table 1. Characteristics of the two study sites

	Urban site	Non-urban site
Catchment area (km ²)	67.3	43.5
Latitude, Longitude	38°03'02.34"S, 145°21'53.42"E	38°0'38.35"S, 145°23'1.32"E
Total imperviousness surfaces (%)	7.1	4.3
Connected imperviousness surfaces (%) ^a	3.1	0.1
Mean catchment rainfall (mm/year) ^b	969.6	969.6
Reach gradient (%) ^c	0.003	0.001
Sinuosity ^c	1.1	1.3
Entrenchment ratio ^c	1.2	1.9
Mean bankfull depth (m) ^c	1.6	0.84
Mean bankfull width (m) ^c	7.02	4.10
Reach bankfull discharge (m ³ /s)	1.72	0.73
Reach median discharge (m ³ /s)	0.39	0.21
Reach baseflow discharge (m ³ /s)	0.03	0.04
Sediment size (D ₅₀) (mm)	6	3

775 ^a The percentage of total imperviousness surface directly connected to the stream

776 ^b Melbourne Water gauge – 586199 (356116.69E, 5791708.09N)

777 ^c Estimates from topographic survey data

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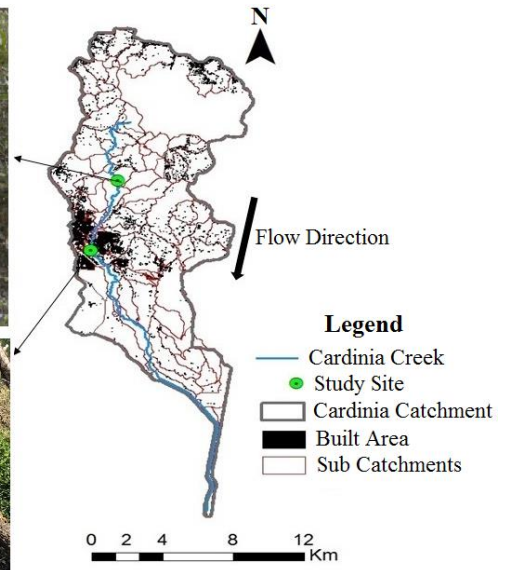
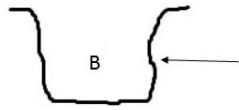
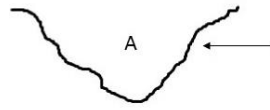
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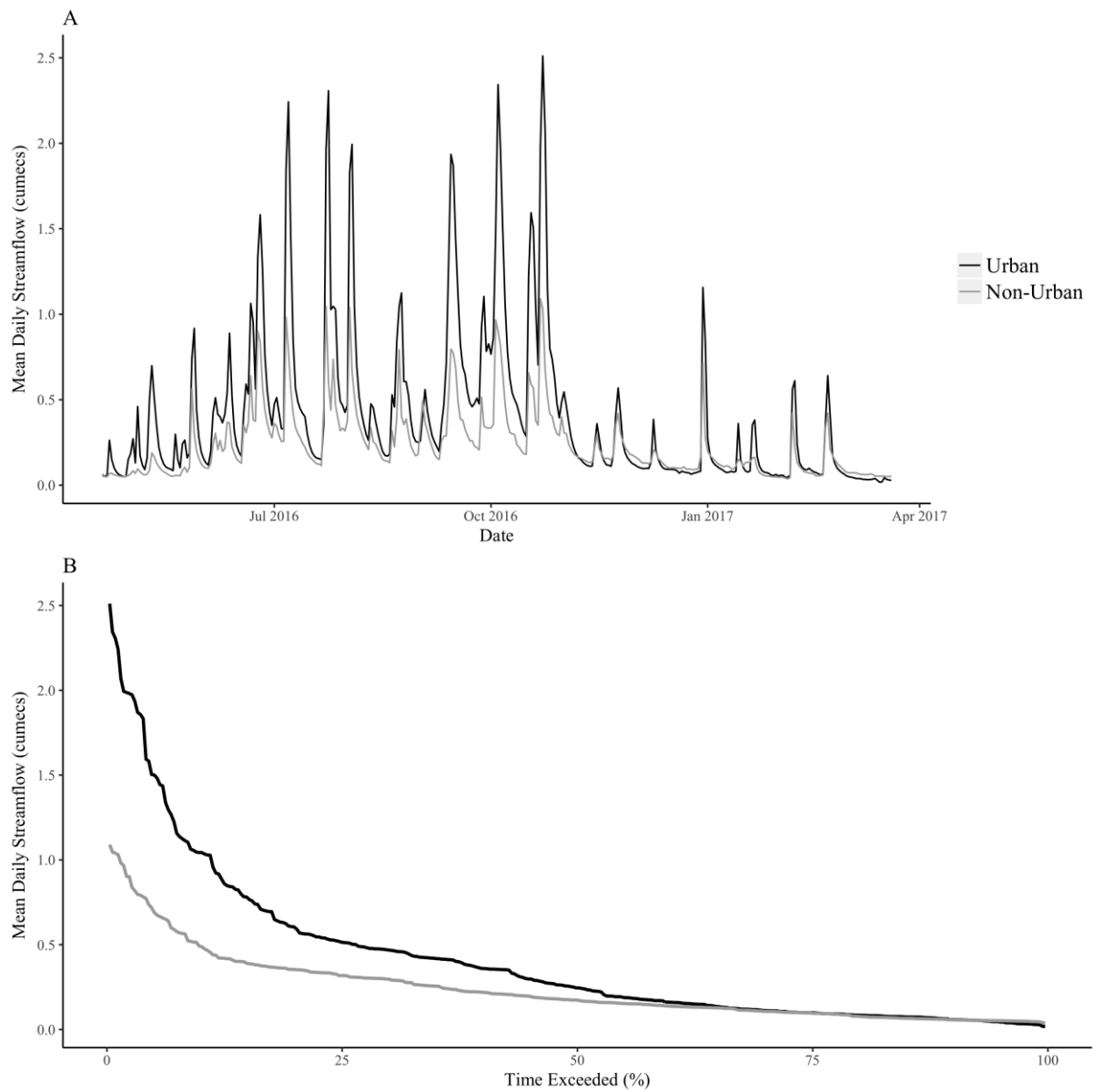
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Typical Channel Cross-Section



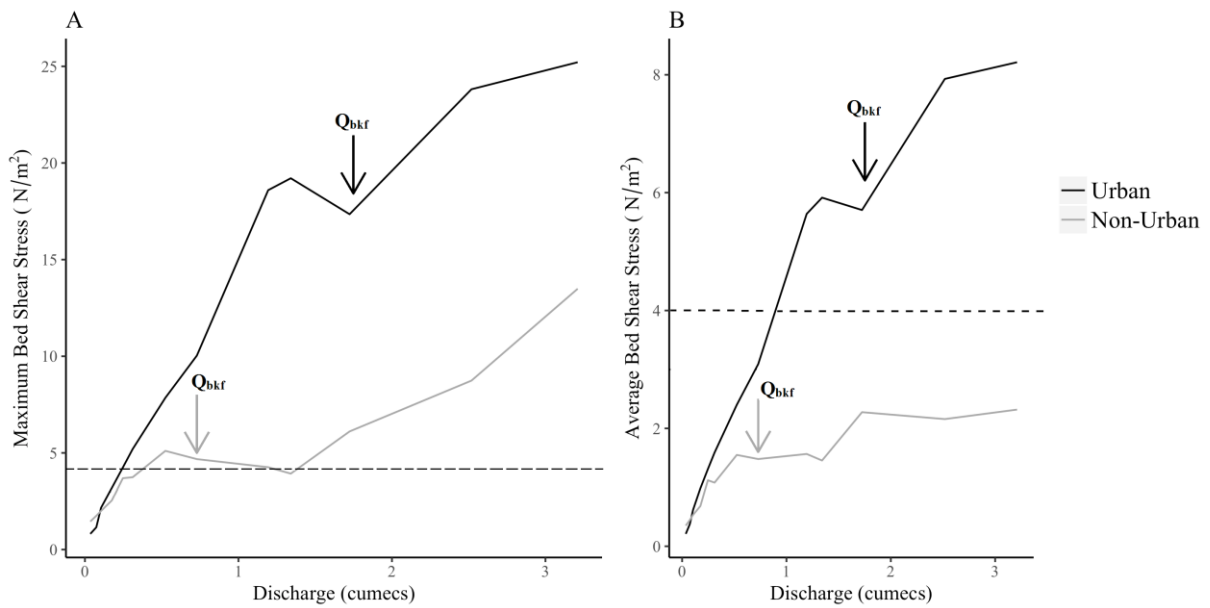
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792 Figure 1. Cardinia Shire catchment, the Cardinia creek and the locations of the (A) non-urban
793 and (B) urban sites



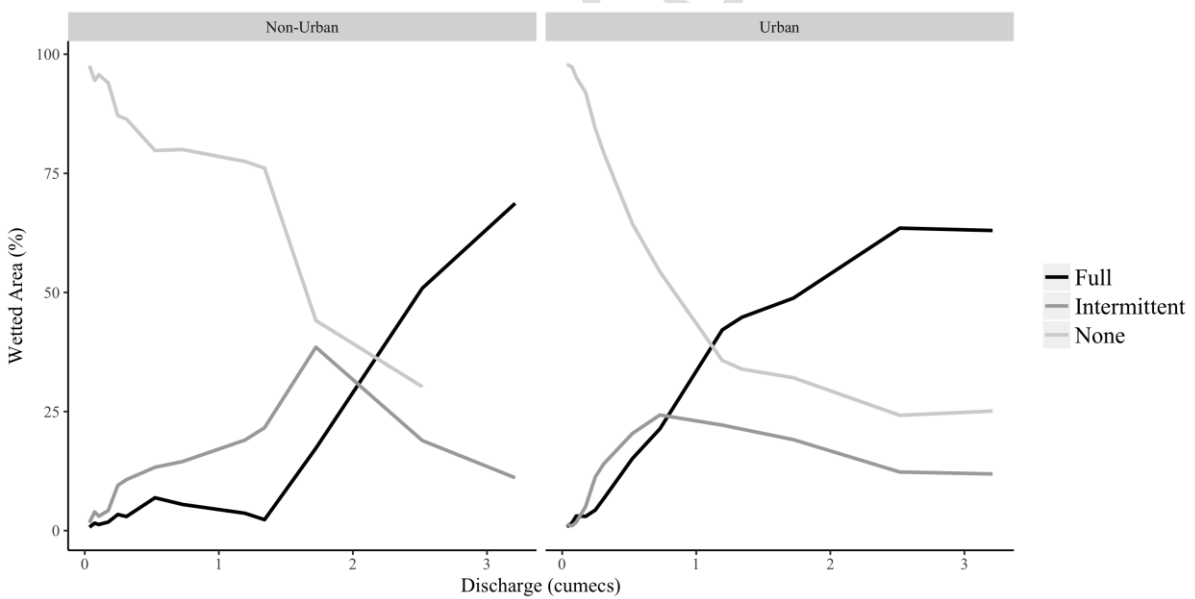
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795 Figure 2. (A) Stream flow hydrographs and (B) Flow duration curve for the urban and non-
 796 urban study sites during study period.



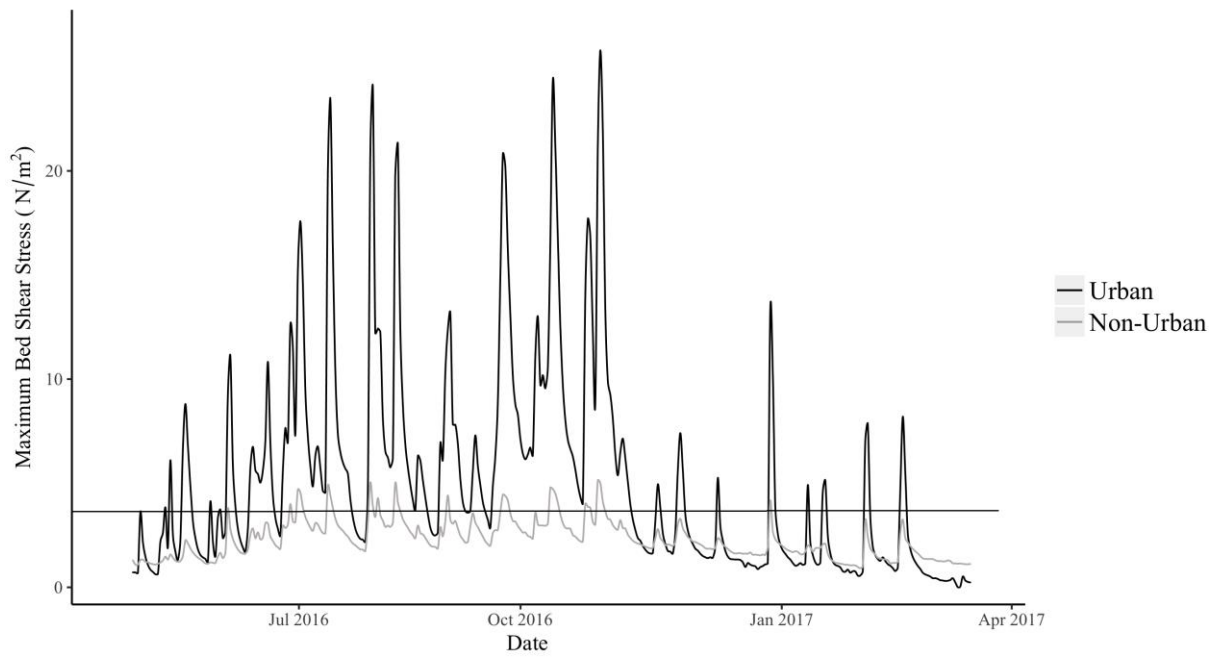
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798 Figure 3. Plot of (A) Maximum (95th percentile) bed shear stress and (B) Average bed shear
 799 stress across selected discharges modelled for the urban and non-urban site. Dashed horizontal
 800 line and the arrows represents the estimated critical bed shear stress and bankfull discharge
 801 respectively.



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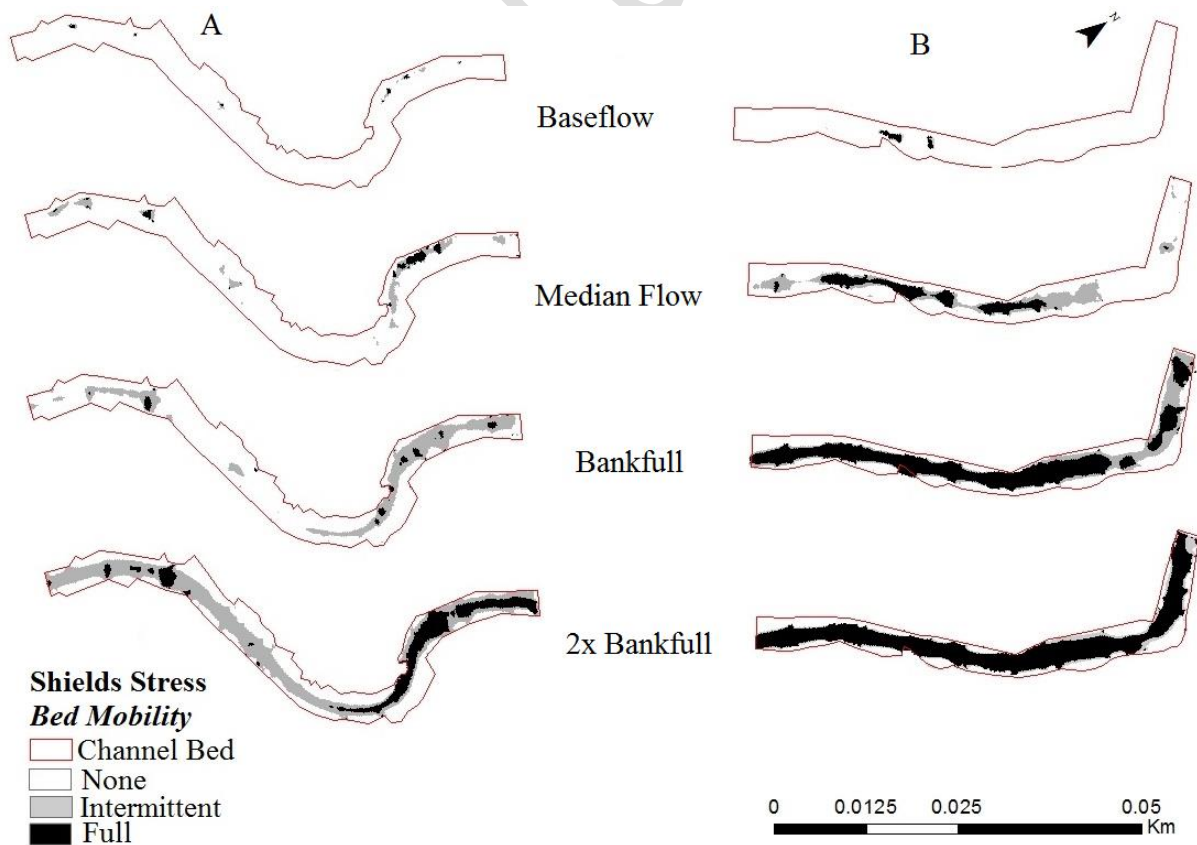
803 Figure 4. Plots of proportion of channel wetted bed area under different Shields stress in non-
 804 urban and urban site as it relates to discharge. Black line represents portion of wetted areas of
 805 bed likely to be entrained with $\tau^* > 0.06$ (Full) whereas deep grey line and light grey line are
 806 areas with τ^* between 0.03 and 0.06 (Intermittent) and $\tau^* < 0.03$ (None) than representing
 807 intermittent and no entrainment bed areas respectively.



808

809 Figure 5. Time series of the daily maximum (95th percentile) bed shear stress for the urban and
 810 non-urban site for the period under investigation. Solid horizontal line represents the estimated
 811 critical bed shear stress.

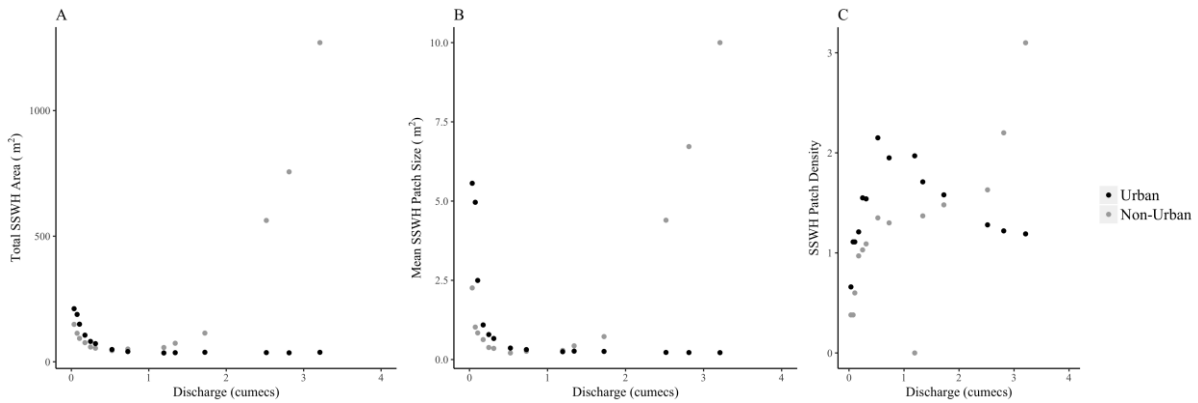
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813

814 Figure 6. 2D hydrodynamic modelling results of Shields stress pattern for selected flows,
 815 showing the mapped distribution of potential bed entrainment for both (A) non-urban and (B)
 816 urban reach. These are representative of the respective Baseflow, Median Q, Qbkf and 2x Qbkf
 817 of the flow regime at both sites.

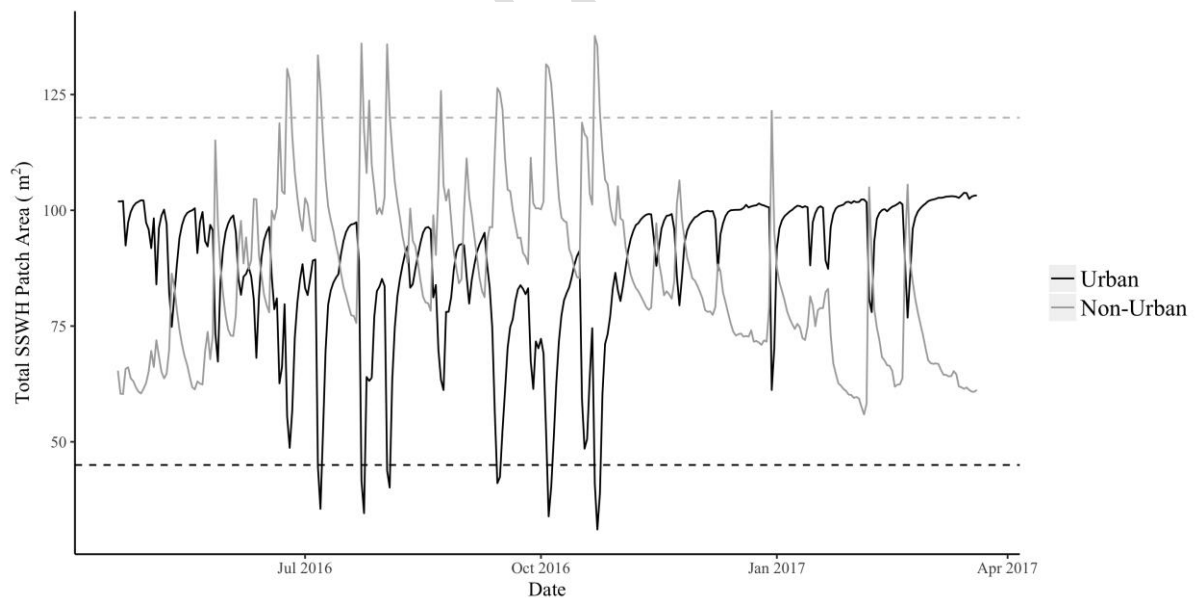
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820 Figure 7. Relations between discharge and selected spatial metrics per 100m of study reach for
 821 Shallow Slow Water habitat (SSWH): (A) Total SSWH patch area, (B) Mean SSWH patch
 822 size, (C) SSWH patch density for urban and non-urban site at selected discharges.

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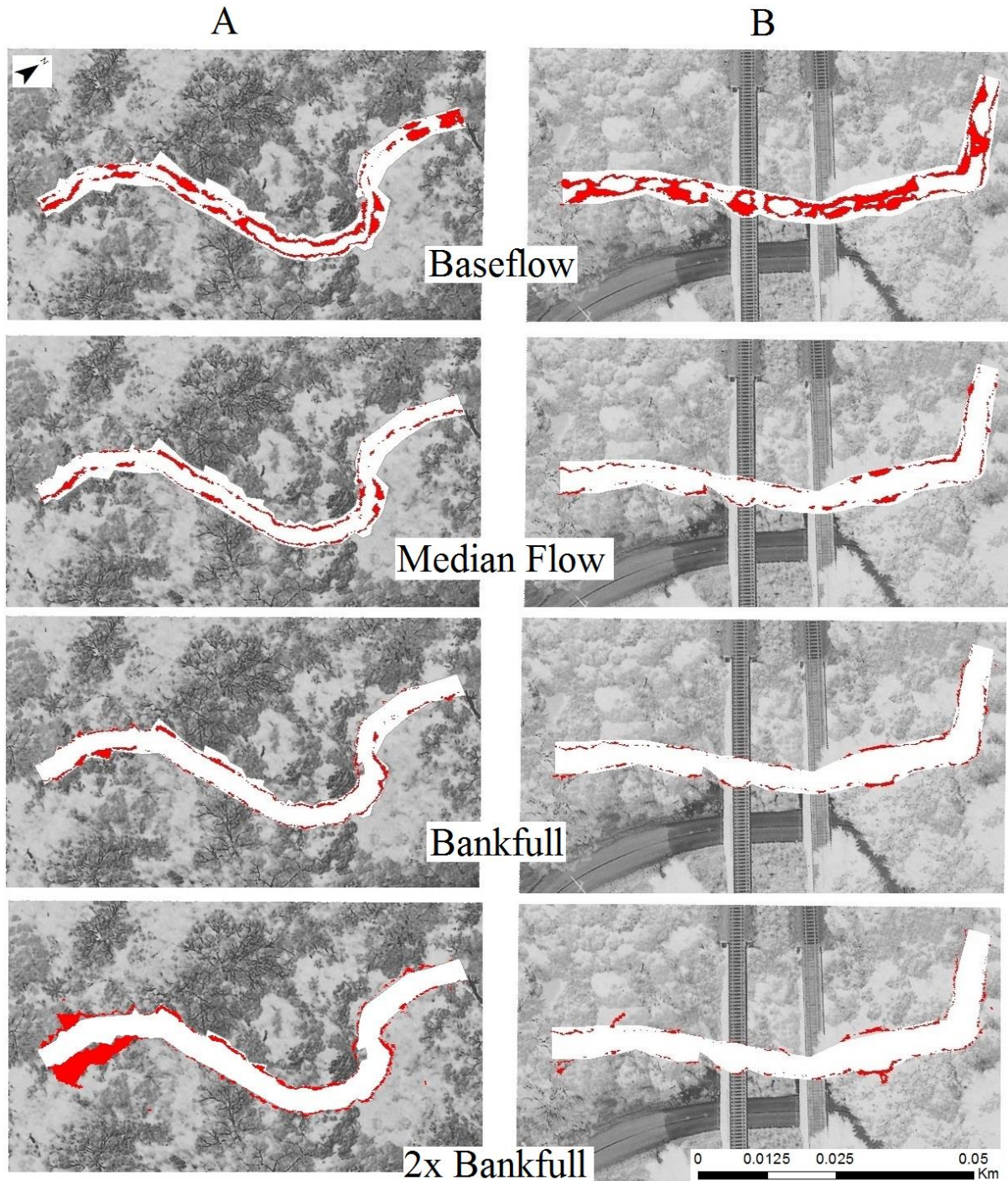


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825 Figure 8. Time series of the daily total SSWH patch area available for the urban and non-urban
 826 site for the period under investigation. Horizontal dashes represent the floodplain level of total

827 SSWH patch area for both non-urban ($SSWH \geq 120m^2 / 100m$) and urban ($SSWH \leq 45m^2 /$
828 $100m$) respectively.

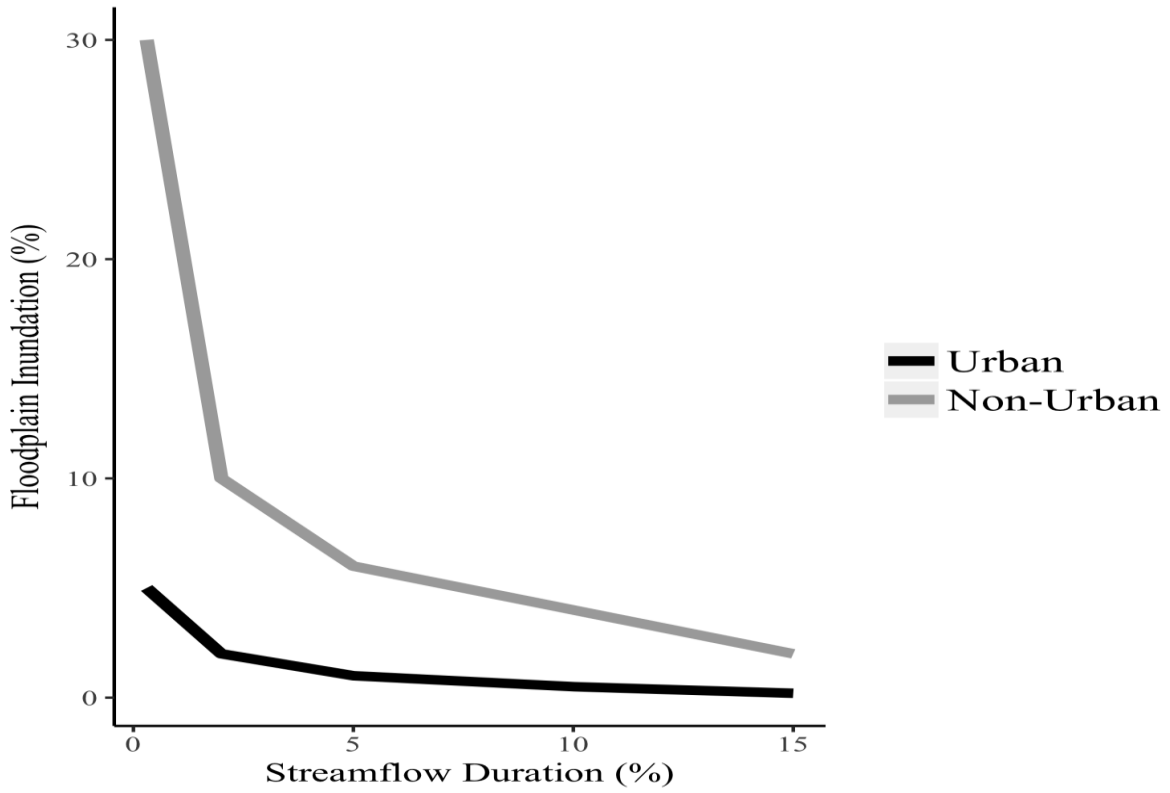
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831 Figure 9. 2D mapped spatial distribution of the SSWH (red shading) for selected discharges in
832 the non-urban (left maps) and urban (right maps) sites. These are representative of the
833 respective Baseflow, Median Q, Qb_{kf} and 2x Q_{bkf} of the flow regime at both sites.

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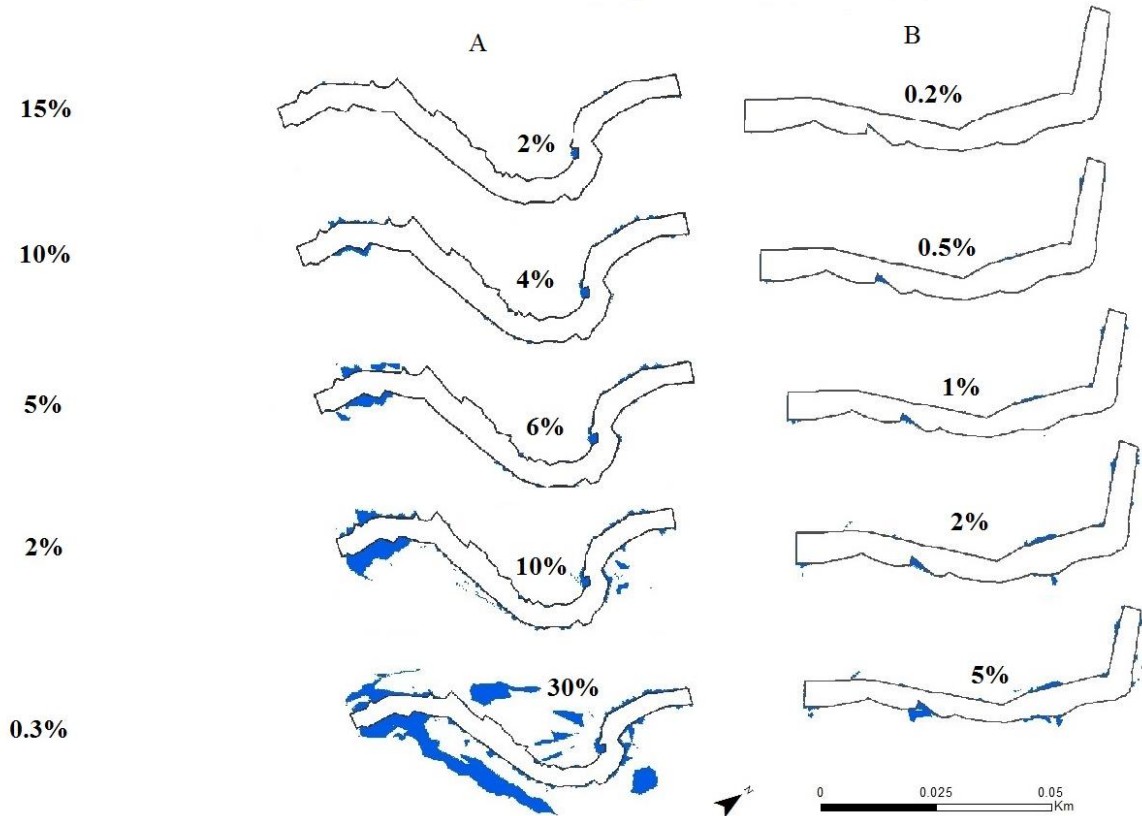
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836 Figure 10. Relation between percentage of floodplain inundation area and selected stream flow
837 duration.

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Streamflow Duration (%)

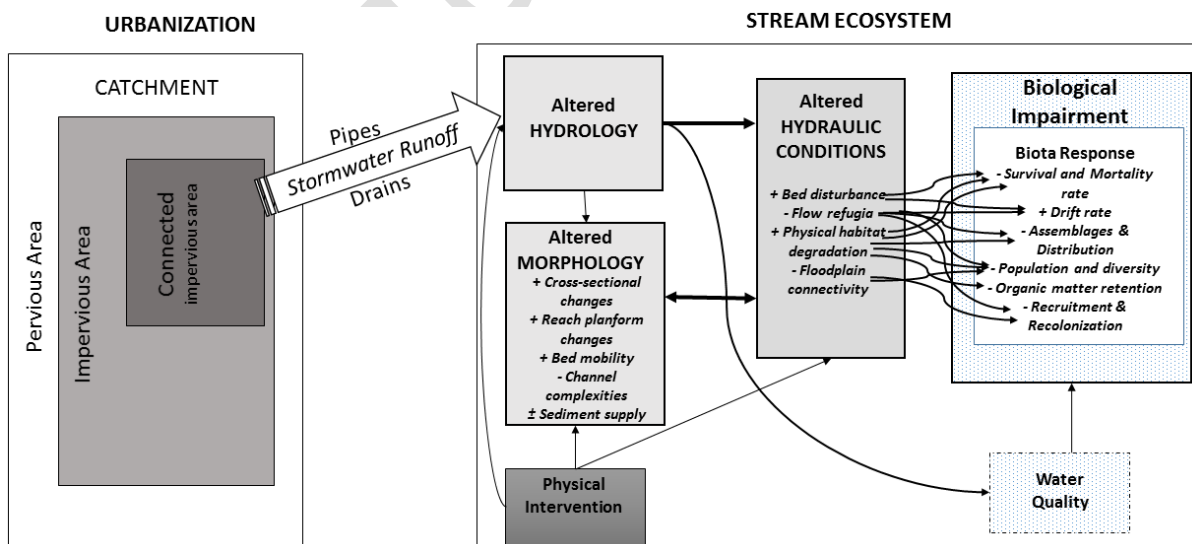
Floodplain Inundation Area (%)



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840 Figure 11. 2D mapped extent of the floodplain inundation area (blue) for selected streamflow
841 durations in the (A) non-urban and (B) urban sites.

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843

844 Figure 12. Conceptual framework of urban impacts on a stream (adapted from Walsh et al.,
845 2005b). All stream ecosystem variables are grouped in one entity (right) and the catchment
846 variables (left). The arrows connecting the different entities shows hypothesized causal

847 relationships and major pathways. While the sources of impact are numerous, the major
848 pathway of changes is stormwater runoff from connected impervious surfaces introduced into
849 the stream by pipes and drains. It also shows that hydraulics is the mediator between exogenous
850 drivers and ecological responses. The direction of the expected effects are by + and – which
851 indicates increasing and decreasing impact respectively.

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accepted corrected