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1 **How alternative urban stream channel designs influence ecohydraulic conditions**

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26 **ABSTRACT**

27 Streams draining urban catchments ubiquitously undergo negative physical and ecosystem  
28 changes, recognized to be primarily driven by frequent stormwater runoff input. The common  
29 management intervention is rehabilitation of channel morphology. Despite engineering design  
30 intentions, ecohydraulic benefits of urban channel rehabilitation are largely unknown and likely  
31 limited. This investigation uses an ecohydraulic modelling approach to investigate the  
32 performance of alternative channel design configurations intended to restore key ecosystem  
33 functioning in urban streams. Channel reconfiguration design scenarios, specified to emulate  
34 the range of channel topographic complexity often used in rehabilitation are compared against  
35 a reference 'natural' scenario using ecologically relevant hydraulic metrics. The results showed  
36 that the ecohydraulic conditions were incrementally improved with the addition of natural  
37 oscillations to an increasing number of individual topographic variables in a degraded channel.  
38 Results showed that reconfiguration reduced excessive frequency of bed mobility, loss of  
39 habitat and hydraulic diversity particularly as more topographic variables were added.  
40 However, the results also showed that none of the design scenarios returned the ecohydraulics  
41 to their reference conditions. This indicates that channel-based restoration can offer some  
42 potential changes to hydraulic habitat conditions but are unlikely to completely mitigate the  
43 effects of hydrologic change. We suggest that while reach-scale channel modification may be  
44 beneficial to restore urban stream, addressing altered hydrology is critical to fully recover  
45 natural ecosystem processes.

46 **Keywords:** Urbanization; Stream restoration; Channel rehabilitation; Hydraulic modeling;  
47 Stormwater; Hydraulics

## 48 **1. Introduction**

49 Urban landuse changes and especially stormwater management are widely recognized as a  
50 driver of major changes in stream ecosystems (Ladson et al., 2006; Fletcher et al., 2014). Well-  
51 documented changes includes substantial hydrological disturbance (characterised by increased  
52 frequency, magnitude and duration of peak flows) (Konrad & Booth, 2005), water quality  
53 disturbance (Brabec et al., 2002), as well as channel morphology degradation (Vietz et al.,  
54 2014), primarily driven by urban stormwater runoff (Walsh et al., 2012). These changes lead  
55 to ecological degradation (Walsh et al., 2005; Paul & Meyer, 2008). As a result, urban streams  
56 are targeted worldwide and there are increasing restoration measures employed by managers  
57 to curb the urban-induced impacts. These measures aim primarily to restore stream biodiversity  
58 and ecological function (Wohl et al., 2005; Bernhardt & Palmer, 2007).

59 Restoration of urban streams generally has two main levers: addressing the altered hydrology  
60 (Burns et al., 2013; Bell et al., 2016) or the degraded channel morphology (Roni et al., 2008;  
61 Chin & Gregory, 2009). Regardless of restoration strategy, addressing channel morphology  
62 degradation remains one of the most common motivations for undertaking stream ecosystem  
63 restoration (Findlay & Taylor, 2006; Jähnig et al., 2009; Palmer et al., 2014). This is  
64 particularly due to the negative impacts of physical degradation on the environmental and  
65 social values that urban streams provide (Elmqvist et al., 2015; Arnold & Toran 2018). As a  
66 result, a majority of management strategies target in-stream morphological reconfigurations  
67 despite their high cost (Montgomery, 2006; Bernhardt & Palmer, 2011; Hering et al., 2015).

68 Approaches to addressing urban stream channel changes have evolved from traditionally  
69 focusing on increasing channel stability and simplification in support of flood control and bank  
70 erosion protection, to now adopting morphological reconfiguration and hydraulic structure  
71 addition in support of improved biodiversity and ecosystem services (Bernhardt et al., 2005;

72 Muhar et al., 2016). For example, morphological naturalization, involving the introduction of  
73 specific instream landforms to have a more natural appearance is widely performed (Sear et  
74 al., 2000; Bernhardt & Palmer, 2011). This usually involves some form of modification of the  
75 longitudinal and cross-section of channel at reach-scale to improve topographic variability  
76 (Sear & Newson, 2004; Wheaton et al., 2004; Pasternack, 2008). These are often done to create  
77 morphological complexity assumed to have the potential to promote ecological improvement  
78 and biodiversity (Chin & Gregory, 2009; Palmer et al., 2010). This assumption is hinged on  
79 research showing that biota richness and diversity and channel topographic heterogeneity are  
80 positively correlated (Brown, 2003; Violin et al., 2011).

81 However, in recent times, concerns over the performance of channel reconfiguration actions to  
82 achieve restoration goals have been raised (Miller & Kochel, 2010; Wohl et al., 2015). Notably,  
83 studies evaluating post-restoration projects have reported they usually yield little or no  
84 ecological benefits (Gurnell et al., 2007; Kondolf et al., 2007; Baldigo et al., 2010; Bernhardt  
85 & Palmer, 2011; Kim et al., 2019), especially for streams draining substantially urbanized  
86 catchments (Walsh et al., 2012). What is missing from the literature is a clear link between  
87 driving topographic and hydrologic factors and resulting ecological outcomes. The missing  
88 link is the domain of ecohydraulics, which explores the mechanisms (herein the interactions  
89 between flow regimes and the channel morphology) and describes hierarchically nested aquatic  
90 and riparian biotic phenomena (Casas-Mulet et al., 2016; Kuriqi & Ardiçlioglu, 2018).

91 To get at the ecohydraulics involved in urban stream syndrome, Anim et al. (2018a) quantified  
92 the hydraulic conditions in urban streams (with altered hydrology) and demonstrated that they  
93 are substantially altered compared to a reference 'natural' stream. The urban stream subjected  
94 to altered hydrology experienced significant increased bed disturbance (bed particle  
95 mobilization), decreased refuge habitat and decreased hydrological connectivity (Anim et al.,  
96 2018a). Whilst most studies evaluating the performance of the urban stream channel

97 reconfiguration outcomes do not report the mechanism leading to failure, the findings of Anim  
98 et al. (2018a) highlight the real issue behind the syndrome itself and restoration failure could  
99 be the altered ecohydraulic conditions. This could be a limiting factor for the lack of desired  
100 ecological improvement. Indeed, it is argued that restoration strategies should consider  
101 hydrogeomorphic process that are directly linked to the ecosystem functioning needs of the  
102 target stream (Wohl et al., 2015). It is important that the channel rehabilitation efforts achieve  
103 the hydraulic habitat conditions that will promote ecological benefits. Hydraulic conditions  
104 influence biota and ecosystem functioning and it is often used to speculate the mechanism that  
105 influence ecological health of streams (Jowett, 2003; Mérigoux & Dolédec, 2004; Clark et al.,  
106 2008; Turner & Stewardson, 2014).

107 In light of the failures of current stream engineering practices, research has called for a move  
108 away from channel-based restoration approach towards addressing the root causes that  
109 fundamentally alters the hydrology and sediment supply (Walsh et al., 2012; Vietz et al., 2016).  
110 However, while addressing the root causes of urban stream syndrome is certainly important,  
111 Anim et al. (2018b) found that once the channel morphology has been substantially degraded,  
112 mitigating altered hydrology alone cannot return ‘natural’ channel ecohydraulics. They  
113 suggested that in such cases, opportunities for channel morphologies rehabilitation may need  
114 to be considered hand-in-hand with addressing catchment drivers (Anim et al., 2018b).

115 In this study, we build on recent findings to explore the research question: ‘How do alternative  
116 channel rehabilitation designs using an increasing number of oscillating topographic variables  
117 impact instream hydraulic conditions?’ We explored the effectiveness of different channel  
118 reconfigurations common to emerging stream channel rehabilitation design concepts (Brown  
119 et al., 2016) on modifying ecologically relevant hydraulic conditions. For each reconfiguration,  
120 we used two-dimensional (2D) hydraulic modelling to quantify changes in bed mobility,  
121 hydraulic diversity and habitat availability. We demonstrate that rehabilitation could support

122 ecosystems through reinstating appropriate hydraulic conditions by means of channel  
123 modification with linked oscillating topographic variables, in addition to modifying flow. By  
124 focusing on how channel morphology relates to hydraulic conditions at an ecological relevant  
125 scale, the opportunities for stream rehabilitation could be made more strategic.

## 126 **2. Methods**

### 127 **2.1. Experimental design**

128 The modelling approach was fourfold (Figure 1). First, we adopted pre-existing case-study  
129 stream reaches selected to physically represent and compare an urban and natural (reference)  
130 setting (with representative hydrology and channel form). Second, a set of synthetic stream  
131 corridor Digital Terrain Models (DTMs) was generated by applying the synthetic river valley  
132 procedure of Brown et al. (2014) using channel parameters data from both real reaches. From  
133 an initial simple synthetic urban channel reach, four different DTMs were created representing  
134 channel reconfiguration designs with incrementally more variables (i.e., depth, width, and  
135 centreline) given natural undulations. These incrementally reconfigured topographic surfaces  
136 of the degraded urban channel characterised different degrees of reach-scale morphological  
137 complexity to mimic the natural 'reference' condition at the reach-scale. Note that for this  
138 study, the channel design focused on reach-scale design excluding local hydraulic structures.  
139 There are too many possible structures one might add to the test scenarios as well as infinite  
140 options for placement position, size, and orientation. That would require a comprehensive study  
141 of its own, which was beyond the scope of this study. Third, a 2D hydraulic model was used  
142 to simulate ecohydraulic impacts of each channel scenario. Finally, the temporally varying  
143 hydraulic performance of each reconfigured channel was quantitatively evaluated using metrics  
144 of known ecological relevance that evaluates the bed disturbance, habitat value and ability to  
145 produce hydraulic diversity. We tested how closely each hydraulic metric deviated from the

146 urban case after channel reconfiguration. These steps are described in more detail in the  
147 following sections.

## 148 **2.2. Study-site settings**

149 The study sites setting used in here were segments of the Cardinia Creek length in the Cardinia  
150 Shire catchment, south-eastern Melbourne, Australia investigated in previous study by Anim  
151 et al. (2018b). The two reaches have distinguished hydrology and morphology, physically  
152 representing an urban and natural settings. The urban reach drains an urbanized section of the  
153 catchment that retains about 40% forest/tree cover, with the remainder of the surface area  
154 cleared for urban development. Some 7% of the total catchment area is impervious with half  
155 of the impervious surfaces connected to the stream through stormwater drainage systems. This  
156 suggests that this reach will be significantly influenced by the catchment land use and upstream  
157 drainage area (Burns et al., 2012). The natural reach drains 50% forest/tree cover and 43%  
158 pasture/grassland cover. 4% of the catchment is covered by impervious surfaces, with only  
159 0.1% draining directly to the stream suggesting minor hydrological disturbance (Walsh, 2004).  
160 Both sites have similar rainfall pattern, averaging ~950 mm/year annually, well distributed over  
161 the catchment, with higher rainfall in winter-spring (Anim et al., 2018a).

### 162 **2.2.1. Study reach topography**

163 The natural reach has an intact and complex naturally meandering, pool-riffle channel  
164 morphology with a sand-gravel bed and lateral benches. The urban reach has an incised  
165 (deepened and widened) and simplified (homogenous) sand-gravel plane bed channel  
166 morphology with less complexity both in cross-profile and planform. Existing field data from  
167 a detailed channel survey of each reach provided typical reach-average channel geometric  
168 elements including bankfull depth ( $H_{bf}$ ), width ( $W_{bf}$ ), slope ( $S$ ) and a representative median  
169 particle size ( $D_{50}$ ).



170 **2.2.2. Hydrological regime**

171 Continuous streamflow gauge records (January 2008- December 2016) providing a good  
172 representation of a typical dry, normal and wet water year conditions were available for the  
173 study reaches. These were that used by Anim et al. (2018b) (Figure 2). The urban streamflow  
174 regime is characterised by an increased frequency of flashy (including higher peak magnitude,  
175 frequency and short-lived) flows occurring especially during winter periods and lower  
176 baseflows during summer compared to the natural. This reflected a typical urban stream  
177 hydrological regime influenced by stormwater runoff from connected impervious surfaces  
178 contributing flows (Burns et al., 2012).

179 **2.3. Synthetic test channel morphology**

180 Archetypal stream channel morphology were created using an open source “RiverBuilder” R  
181 package (version 0.1.0) which is an emerging technique of synthesizing channel topography  
182 for science and engineering application (Arroyo & Pasternack, 2017). RiverBuilder as a  
183 practical river design tool is based on the synthetic river valley framework of Brown et al.  
184 (2014) that renders a DTM from user-selected geometric functions describing the topographic  
185 variability at reach and subreach scales. Herein we provide only the equations used to create  
186 the specific DTMs used in this study.

187 **2.3.1. Channel design parameterization**

188 RiverBuilder allows synthetic channel topography to be developed based on the following  
189 reach-average input dimensions:  $H_{bf}$ ,  $W_{bf}$ ,  $S$  and  $D_{50}$ , floodplain width and slope. These inputs  
190 were computed and scaled from surveying the case study reaches (Table 1). From these inputs,  
191 user-defined subreach-scale topographic variability can be added using combinations of  
192 geometric functions,  $f(x_i)$  in RiverBuilder. There is no limit to how many different functions  
193 may be added together to represent the longitudinal structure of an individual geometric

194 variable. The subreach variability for each channel was designed using Eq (1) and (2) such that  
195 the local bankfull width and bed elevation of thalweg was calculated as:

$$W_{bf}(x_i) = (\overline{W_{bf}f(x_i)} + \overline{W_{bf}}) \quad (1)$$

$$z_t(x_i) = (\overline{H_{bf}f(x_i)} + \overline{H_{bf}}) + S(\Delta x_i) + Z_d \quad (2)$$

196 where  $W_{bf}(x_i)$  and  $z_t(x_i)$  are the bankfull width and local bed elevation at position  $x_i$   
197 respectively, and  $Z_d$  is the user-defined datum. There are many possible functions,  $f(x_i)$   
198 provided in RiverBuilder including linear, trigonometric and Perlin noise that can be used to  
199 describe the channel variability and for each an infinite variety are obtainable depending on  
200 chosen parameters (Brown et al., 2014). Herein, the general sinusoidal model was used to  
201 achieve the variability of  $W_{bf}$  and  $Z_t$  about the reach-averaged values by a control function  
202  $f(x_i)$  nested in Eqs. 2 and 3 as

$$y(x_i) = a_s \sin(b_s x_r + \theta_s) \quad (3)$$

203 where  $y_i$  is the dependent control function values,  $a_s$ ,  $b_s$ , and  $\theta_s$  are the amplitude, angular  
204 frequency and phase for the sinusoidal competent and  $x_r$  is the Cartesian stationing in radians  
205 (Brown et al., 2014). The channel reach-average and variability geomorphic attributes used in  
206 the design of the synthetic DTMs of each investigate channel configurations are shown in Table  
207 1.

208 Table 1.Reach average and control functions parameters used for each designed channel  
 209 scenario.

<b>Reach channel parameters</b>		<b>Urban</b>	<b><math>Urb_W</math></b>	<b><math>Urb_D</math></b>	<b><math>Urb_{W+D}</math></b>	<b><math>Urb_{W+D+M}</math></b>	<b>Natural</b>
Bankfull width (m)	$W_{bf}$	6.50	7.29	6.50	6.47	6.50	4.2
Bankfull depth (m)	$H_{bf}$	0.97	0.90	0.89	0.92	0.75	0.6
Median particle size (m)	$D_{50}$	0.006	0.006	0.006	0.006	0.006	0.006
Channel Slope (%)	$S$	0.002	0.002	0.002	0.002	0.002	0.001
Vertical datum (m)	$Z_d$	1000	1000	1000	1000	1000	1000
Channel length (m)	$L_x$	150	150	150	150	150	150
Sinuosity	$S_L$	1.0	1.0	1.00	1.00	1.20	1.30

210

<b>Variability parameters</b>		<b>Urban</b>	<b><math>Urb_W</math></b>	<b><math>Urb_D</math></b>	<b><math>Urb_{W+D}</math></b>	<b><math>Urb_{W+D+M}</math></b>	<b>Natural</b>
<b>Bankfull width</b>	$a_s$	0	0.25	0	0.25	0.25	0.25
	$b_s$	0	3	0	3	3	3

	$\theta_s$	0	0	0	0	0	0
<b>Bed elevation</b>	$a_s$	0	0	0.25	0.25	0.5	0.5
	$b_s$	0	0	3	3	3	3
	$\theta_s$	0	0	0	0	0	0
<b>Planform</b>	$a_s$	0	0	0	0	0	10
	$b_s$	0	3	3	3	3	1
	$\theta_s$	0	0	0	0	0	0
<b>Floodplain outline</b>	$a_s$	0	0	0	0	5	0.25
	$b_s$	0	0	0	0	1	2
	$\theta_s$	0	0	0	0	3.14	3.14

211

### 212 **2.3.2. Channel design configurations.**

213 The synthetic channel of the urban and natural reach of the case-study settings was first  
214 developed using the reach and sub-reach channel parameters. From the single synthetic channel  
215 reach developed for the urban reach (*Urb*), four different DTMs were created representing  
216 channel restoration design with variability that spans the full domain of bed and width  
217 undulation combinations (Table 2). Here, each channel reconfiguration created is analogous to  
218 some typical channel designs employed by practitioners to enhance channel morphology. For  
219 example, bed undulations are commonly used without width undulations. Meanwhile, width  
220 undulations are increasingly recognized as important hydraulic controls and are beginning to  
221 show up in urban stream restoration projects. The first channel reconfiguration scenario is the  
222 urban channel with added width variation only (*Urb<sub>W</sub>*). The second scenario is urban channel  
223 with added depth variation only (*Urb<sub>D</sub>*). The third is urban channel with both width and depth  
224 variation (*Urb<sub>W+D</sub>*). In this case, the two variations are linked with a positive geomorphic  
225 covariance structure (i.e. high, wide riffles and narrow, deep pools) typical of self-sustainable

226 riffle-pool systems (Brown & Pasternack, 2017). The fourth is urban channel with positively  
 227 co-varying width and depth undulations as well as meandering (sinuosity) ( $Urb_{W+D+M}$ ). In this  
 228 study, the same reach-average input values were used for the pre-restored and restored  
 229 configurations of the urban channel. In additions, bed material is kept uniform for all channels.  
 230 See Supplementary Material for full topographic surfaces of designed synthetic channels.

231 Table 2. Channel morphological designs scenarios investigated in this study. Channel  
 232 archetype are in order of morphological complexity (from least to more complex) condition  
 233 compared with the reference ‘natural’ channel condition. Subscripts W, D and M represents  
 234 width, depth and meander channel features respectively.

<b>Channel archetype Scenario</b>	<b>Description and geomorphic elements included</b>	<b>Design conceptualization analogous</b>
<b>urban channel (<math>Urb</math>).</b>	Semi-confined uniform (with no width and depth undulation) channel	Channelized and greatly morphologically altered channel with uniform cross-sections and longitudinal slope
<b><math>Urb_W</math></b>	Urban channel with only width undulation	Approach analogous to local widening to allow channel movement within limited area
<b><math>Urb_D</math></b>	Urban channel with only depth undulation	Approach analogous to reconfiguring incised channels with undulating streambed resembling pool-riffle sequence which is expected to offer higher degree of ecological function

<b><i>Urb<sub>W+D</sub></i></b>	Urban channel with both width and depth undulation	Approach comparable to local widening with undulating streambed similar to pool-riffle sequence
<b><i>Urb<sub>W+D+M</sub></i></b>	Urban channel with both width and depth undulation and meanders	Naturalised morphology, close to typical natural channel (channel with more varying topographic landforms)
<b>Natural channel (Nat).</b>	Bed and width varying with meanders and lateral benches	complex varying cross-sections, sinuous pool-riffle channel morphology with lateral benches, local topographic perturbations

235

#### 236 **2.4. 2D Hydraulic modeling**

237 2D hydraulic modeling was undertaken using the TUFLOW Classic model (Build 2016 0-  
238 3\_w64) that solves the full 2D, depth-averaged momentum and continuity equations for free  
239 surface flow equations. TUFLOW has been extensively used to study variety of  
240 hydrogeomorphic processes and allows a robust 2D modeling of rivers with complex flow  
241 patterns which makes it a suitable computational tool for complex hydraulic characterization  
242 (Syme, 2001). From the DTM data points generated for each channel by RiverBuilder, a square  
243 grid computational mesh was constructed with 150 longitudinal nodes spaced at 0.3 m. The  
244 default TUFLOW Smagorinsky viscosity was used for turbulence closure with coefficient  
245 value of 0.5 and constant value of 0.005 m<sup>2</sup>/s suitable for shallow waters (Anim et al., 2018a).  
246 A Manning's coefficient n value of 0.04 was used, representing typical unvegetated coarse-  
247 particle surface roughness (Arcement & Schneider, 1989).

248 Model simulations used discharge ( $Q$ ) as input and flow stage as the downstream boundary  
249 condition. Discharge and corresponding flow stage were estimated using Manning's equation

250 based on representative cross-sections of the synthetic DTMs (Table 2). Bankfull stage and  
251 wetted perimeter were calculated manually from the cross-sections and cross-sectional area  
252 determined using the parabolic approximation. Discharge ranged from 0.1-1.0x the bankfull  
253 flow ( $Q_{bkf}$ ) stage. The water surface elevation (WSE) at which flow overtops the banks was  
254 the  $Q_{bkf}$  stage. Model outputs include hydraulic rasters of depth-averaged velocity in the  
255 direction of flow, water depth, bed shear stress ( $\tau_b$ ) and WSE. ArcGIS (Esri ArcGIS desktop  
256 10.2) was used to process and analyze these outputs to evaluate each investigated channel  
257 configuration. Typical of published exploratory numerical modeling studies, calibration of bed  
258 roughness or eddy viscosity was not possible as the study uses numerical models of theoretical  
259 channel archetypes in purely exploratory mode (Pasternack et al., 2008; Brown et al., 2016;  
260 Lane et al., 2018).

## 261 **2.5. Ecohydraulic metrics**

262 The study explored three ecologically relevant hydraulic characteristics that have been  
263 mechanistically linked with stream ecosystem functions: (i) channel bed disturbance that  
264 impacts bed particle mobilization and disturbance of benthic dwelling biota (Gibbins et al.,  
265 2010); (ii) hydraulic diversity – Hydro-Morphological Index of Diversity (HMID); and refuge  
266 habitat availability - Shallow Slow-Water Habitat (SSWH). They were quantified using related  
267 hydraulic metrics including near-bed Shield stress as indicators of bed mobility, a measure of  
268 flow velocity and depth heterogeneity reflecting the reach hydraulic diversity and a measure of  
269 physical habitat area that determines the availability of slow and shallow depth water  
270 respectively. These hydraulic metrics were determined from the raster outputs of the hydraulic  
271 model calculated using python decision tree in ArcGIS over defined threshold bounds.

### 272 **2.5.1. Bed disturbance**

273 Frequent bed disturbance increases channel instability and degradation and also drift of biota  
274 that lives in them (Hawley et al., 2016; Lobera et al., 2017). Non-dimensionalized bed shear

275 stress, Shields stress ( $\tau^*$ ) was used to quantify and compare each channel for their bed  
276 mobilization potential. This was estimated in each grid cell of the model grid cell as:

$$\tau^* = \frac{\tau_b}{D_{50}(\gamma_s - \gamma_w)} \quad (4)$$

277 where  $\gamma_s$  and  $\gamma_w$  are the unit weight of bed particle and water respectively and  $\tau_b$  is bed  
278 shear stress. Herein, a critical entrainment threshold ( $\tau_c^*$ ) of 0.045 (Lisle et al., 2000; Sawyer  
279 et al., 2010) was used to differentiate the portions of the channel bed that indicate mobility ( $\tau^*$   
280  $> \tau_c^*$ ) and stable ( $\tau^* < \tau_c^*$ ).

### 281 **2.5.2. Hydraulic diversity**

282 The channel diversity of flow velocity and depth is well recognized as an essential element of  
283 ecosystem health supporting various life history strategies of biota (Verberk et al., 2008;  
284 Rosenfeld et al., 2011). We estimated this hydraulic heterogeneity using the hydro-  
285 morphological index of diversity (HMID) developed by Gostner et al. (2013). HMID quantifies  
286 the overall hydraulic diversity in the channel for a given discharge based on the reach-scale  
287 coefficient of variation (CV) of flow velocity ( $u$ ) and water depth ( $d$ ) as:

$$HMID_{channel} = (1 + CV_u)^2 + (1 + CV_d)^2 \quad (5)$$

288 where  $CV = \sigma/\mu$ ,  $\sigma$  and  $\mu$  are the standard deviation and mean value respectively. HMID  
289 values were classified to reflect by Gostner et al. (2013) such that  $HMID < 5$  assumes low  
290 diversity;  $5 < HMID < 9$  assumes medium or transitional diversity;  $HMID > 9$  assumes high  
291 diversity.

### 292 **2.5.3. Refuge habitat availability**

293 SSWH is critical to biota that depend on them as refugia particularly during flash flood as well  
294 as serving as rearing and breeding habitat, and promoting organic matter retention (Schiemer  
295 et al., 2001; Vietz et al., 2013). Herein, the relative refuge habitat availability was examined



296 by estimating the SSWH area. SSWH was calculated from the flow depth and velocity model  
297 output using an ArcGIS python script that processes water depth and velocity raster outputs to  
298 locate cells with joint velocity and depth values of 0-0.2 m/s and 0-0.3 m respectively. This  
299 depth and velocity criteria is reported to be preferred by fish (Milhous & Nestler, 2016) and  
300 benthic macroinvertebrates (Shearer et al., 2015) in streams.

## 301 **2.6. Hydraulic response analysis**

302 To initiate a comparative analysis among the different channel configurations, first a functional  
303 relationship was developed for the range of simulated flows for each hydraulic metric. This  
304 relationship was then integrated with the hydrological time series to achieve hydraulic metric  
305 time series representing the temporal pattern of the hydraulic response under each channel. The  
306 urban hydrological time series was parsed into the functional relationship for the urban and  
307 reconfigured urban channel scenarios. Similarly, the natural hydrological time series was  
308 parsed into that of the natural channel scenario. Then by quantitatively characterizing and  
309 comparing the temporal hydraulic variation, we evaluated the relative influence of the channel  
310 reconfiguration from the pre-restored condition towards the natural conditions. The statistical  
311 analysis of the time series of each metric (mean daily) examined the relative percent change of  
312 the various aspects of the hydraulic patterns: magnitude, duration and frequency as key element  
313 of the hydraulic template for each scenario. The analysis also accounted for the hydraulic  
314 metric change with flow in relation to defined thresholds. In this study, hydraulic metrics were  
315 considered only for flows up to bankfull.

## 316 **3. Results**

317 Hereinafter, the use of “reference case” and “urban case” scenarios refers to hydraulic  
318 conditions in (i) the natural channel under natural hydrological regime and (ii) unrestored urban  
319 channel under urban hydrological regime respectively.

### 320 3.1. Bed disturbance

321 Results show a general trend of increase of bottom shield stress with increasing discharge with  
322 a rapid increase in the Shield stress values as flow increased under the two urban case scenarios  
323 with no bed undulation (*Urb* and *Urb<sub>W</sub>*) (Figure 3a). The results show that the maximum  
324 bottom shield stress per unit flow decreased as the channel topographic variability increased.  
325 The bed particle mobility threshold was applied to the shield stress results for each reach to  
326 determine the proportion of channel bed area with Shield stress higher than the threshold of  
327 mobility (Figure 3b). It indicates that the increasing number of topographic variables made to  
328 undulate invariably decreased the areas of channel bed experiencing mobility particularly for  
329 *Urb<sub>W+D+M</sub>* and the natural channel morphology. This suggests that morphology with at least  
330 one undulating geometric layer for each topographic variable nested on top of the basic reach-  
331 scale uniform channel template potentially decreased the mean shear stress as flow increases.  
332 This phenomenon was most relevant at discharge stages over  $0.5Q_{bkf}$ . As discharge exceeds  
333  $0.6Q_{bkf}$ , urban channels with only width or depth undulation have less control over bed  
334 mobilization and the whole channel trends towards mobility, similar to the urban channel. For  
335 such high flows, adding both width and depth variability substantially reduced the wetted bed  
336 area experiencing mobility. For these channels, almost 45% of the bankfull channel provided  
337 undisturbed benthic area compared to the plane bed channels.

338 In addition, temporal variability of daily shield stress was greater in the urban plane channel  
339 bed compared to the pool-riffle bed for the studied hydrological period (Figure 4). This was  
340 however dominated by high occurrences of daily Shield stress above threshold for mobility  
341 ( $\tau^* > 0.045$ ) with a median value of 0.042 and 0.038 for *Urb* and *Urb<sub>W</sub>* respectively. This  
342 indicated temporal persistent of unstable channel bed. This frequently occurring case of  
343 mobility was substantially reduced as the topographic complexities of the urban channel  
344 increased particularly. In contrast, temporal variability of daily shield stress for the natural

345 channel scenario showed incremental period of below mobility threshold Shield stress values  
346 with median of 0.026 indicating comparably stable bed.

### 347 **3.2. Hydraulic diversity**

348 The greatest different between the channel scenarios investigated occurred at low flows ( $<$   
349  $0.3Q_{bkf}$ ), where highest HMID values were observed (Figure 5) and decreased with increasing  
350 flow ( $>0.5Q_{bkf}$ ). The low-to-peak flow loss of hydraulic diversity showed the natural channel  
351 maintaining high HMID values where diversity was within moderate to high class for flows up  
352 to  $0.7Q_{bkf}$ . In contrast, HMID values were only within moderate values for urban channel  
353 ( $Urb$ ) even at low flows, which plummeted to low diversity (HMID $<$ 5) as flow exceeds  
354  $0.4Q_{bkf}$ . During the low flows, HMID was almost twice as high in the pool-riffle channel types  
355 compared to the plane bed channels. Whilst HMID decreased with increasing flow, pool-riffle  
356 channel with meandering ( $Urb_{W+D+M}$ ) with more gradual side slopes showed some increases  
357 in HMID as flow exceeded  $0.6Q_{bkf}$ .

358 The HMID was lowest in the channel scenarios with no bed undulation ( $Urb$  and  $Urb_W$ )  
359 (Figure 6), with a narrow range. For all flows, mean velocity in these channels were remarkably  
360 higher than the pool-riffle channels. In contrast, the range of velocity and depth was widest in  
361 the pool-riffle channels with lower minimum and higher maximum values across all modelled  
362 flows. This resulted in higher depth range and CV particularly for  $Urb_{W+D}$  and  $Urb_{W+D+M}$ .  
363 The plane channel bed morphologies showed the least temporal persistence of high hydraulic  
364 diversity (HMID $>$ 9) with a median HMID value of 4.8 and 5.5 for scenarios  $Urb$  and  $Urb_W$   
365 respectively. The limited temporal persistence of high hydraulic diversity was improved by  
366 inclusion of both width and depth variation in the channel ( $Urb_{W+D}$  and  $Urb_{W+D+M}$ ). These  
367 channels mostly experience medium and high diversity particularly for  $Urb_{W+D+M}$  with a  
368 median value of 7.6. The natural case showed temporal persistence of high hydraulic diversity.

### 369 3.3. Refuge habitat availability

370 Similar trend of changes to SSWH availability with flow was observed for all channel scenarios  
371 (Figure 7a and 7b). SSWH area was high at low flows (below  $0.3Q_{bkf}$ ) occupying more than  
372 50% of total wetted area in the reach. The gradually changing morphological relief of the  
373 natural channel maintained more than 50% of total SSWH patch up to  $0.5Q_{bkf}$  and decreased  
374 steadily as flow increased. The plan bed channels ( $Urb$  and  $Urb_W$ ) inundated to higher flow  
375 depths and velocities as flow increased, thus the SSWH area plummeted at rapid rates. SSWH  
376 area was higher in the urban channel with only depth variation ( $Urb_D$ ) than plane bed  
377 morphology at flows up to  $0.5Q_{bkf}$ , beyond which they were nearly equivalent. For each  
378 modelled flow, an average of 15% increase of the SSWH area was observed when both width  
379 and depth variability ( $Urb_{W+D}$  and  $Urb_{W+D+M}$ ) was added to the plane bed channel  
380 morphology (Figure 7b). Here, the proportion of the reach occupied by SSWH area was at least  
381 2x higher than the plane bed channels.

382 The frequently occurring high flows ( $>0.6Q_{bkf}$ ) in the urban hydrology reflected in the high  
383 temporal persistence of smaller SSWH areas ( $< 300m^2/150m$ ) in the urban channels  
384 particularly for the plane bed channels. A median value of  $245.2 m^2/150m$  and  $264.5 m^2/150m$   
385 was observed for  $Urb$  and  $Urb_W$  respectively. This was however greatly improved for the pool-  
386 riffle bed with width variation channel morphologies, with about 50% increase in the median  
387 SSWH values compared to confined plane bed channel. High temporal persistence of larger  
388 SSWH area ( $>500m^2/150m$ ) was observed for the natural channel with a median value of  
389  $456.3m^2/150m$ . This reflected a natural complex morphology engaged by the long duration-  
390 low magnitude flows in the natural hydrological regime with reduced frequency of high flows.

## 391 **4. Discussions**

### 392 **4.1. Hydraulic performance of channel reconfiguration scenarios**

393 Comparison of quantitative hydraulic metrics for each of reconfiguration scenario reveals two  
394 general points. Firstly, simple channel form, defined as a uniform, U-shaped, single-threaded  
395 channel with no width, depth, or centreline variation, leads to simple hydraulics. The simplified  
396 (homogenous) channel topography, typical of many urban settings, deleteriously alters  
397 hydraulic patterns. This is perhaps expected but not necessarily well proven with data as  
398 provided in this study. Secondly, channel forms with increasingly more geometric variables  
399 having undulations yield to more increasing better performing hydraulics. The more geometric  
400 elements were added to the channel up to the full patterning of depth, width, and centreline  
401 structures, the less sensitive the channel was to an altered urban flow regime highlighting the  
402 importance of spatial diversity in channel morphology for supporting stream ecosystem health  
403 (eg., Escobar-Arias & Pasternack, 2010; Schwartz et al., 2015; Lane et al., 2018a). This does  
404 not mean that adding infinitely more geometric functions to any one variable or by adding  
405 many more undulating geometric variables will make the conditions better than what was  
406 studied; it will take more research to figure out what is optimal for each river setting. Channels  
407 with naturalized geometric oscillations coherently phased to yield requisite morphodynamic  
408 processes dynamic morphologies have a better chance of minimizing the influence of altered  
409 hydrological regime on the hydraulic conditions. Thus, making biota less prone to rapid  
410 temporal fluctuations than an unrestored reach.

411 Designing the urban degraded channel to include a pool-riffle sequence, plus some undulations  
412 in width or sinuosity, provides greater opportunity for improved hydraulic conditions. For  
413 instance, in addressing the bed mobility rate, Schwartz et al. (2015) reported that restoring  
414 riffle-pool structure promotes shear stress reversals between low and high flows as well as high

415 flow acceleration and deceleration between pools and riffles (eg., Brown & Pasternack, 2017).  
416 This is essential for spatiotemporal heterogeneity of the hydraulic characteristics of the flow  
417 such as water depth, flow velocity and turbulence, to promote habitat creation and quality  
418 (Clarke et al., 2003). While predicting the ‘optimal’ channel morphology for urban restoration  
419 design is beyond the scopes of the current study, our results suggest that the hydraulic  
420 conditions can be significantly modified with even minor width and depth undulations and  
421 sinuosity patterns. Brown and Pasternack (2014) reported that multiple physical mechanism  
422 process occurs as modulated by the interactions of the flow hydrology with complex channel  
423 topography. It is thought that channels with different topographic features steer the flows in  
424 such a way that different features turn on and off to create diverse patterns of hydraulic  
425 conditions (Strom et al., 2016). This will potentially support sustaining spatial and temporal  
426 hydraulic patterns at levels below the threshold for certain processes (eg., Gostner et al., 2013;  
427 Vanzo et al., 2016; Lane et al., 2018a). For instance, Anim et al. (2018a) found that complex  
428 topographic variability decreased areas of channel bed subjected to high hydraulic stress for  
429 bed particle movement even with increasing flows.

430 To summarize, topographic dynamic channels may support fundamental physical process at  
431 appropriate levels even under altered urban hydrology characterized by increased frequency,  
432 magnitude and volume of storms flows. It is however worth noting that *appropriate* here is  
433 intended to imply reduction in excessive frequency of bed disturbance or scouring rates, loss  
434 of physical habitat and hydraulic diversity.

#### 435 **4.2. Can modifying reaches reverse catchment-scale degradation sources**

436 Results demonstrated that reconfiguring channel morphology close to natural form can help to  
437 accommodate changes to altered flow. Doing so restores ecologically relevant hydraulic  
438 conditions. For example, refuge habitat availability between simple channels and the most  
439 complex improved by 32%. Reinstating bed diversity increases hydraulic diversity by 21%,

440 with a further 20% increase when sinuosity was added. Bed disturbance can be decreased by  
441 45% (see Table 3). Further improvements could potentially be achieved by combinations of  
442 high-undulations and geomorphic covariance structures and through the addition of sub-reach-  
443 scale in-stream features such as alluvial benches, boulder clusters, wood structures, alluvial  
444 steps.

445 However, when compared to the reference case scenario, the hydraulic patterns of modified  
446 channels under degraded hydrology were still not returned to a fully 'natural' condition. In this  
447 light, we argue that attempts to restore extensive morphological features is likely to be  
448 ineffective when a counter fundamental problem like dramatically fluctuating flows remains  
449 unaddressed. Given that the urban hydrology is characterised by increased frequency and  
450 magnitude of peak flows (Walsh et al., 2012), the efficacy of increasing morphological  
451 variability to ensure high diversity hydraulic habitat will be affected.

452 Only 7% of the total catchment area of the urban site is impervious with about half of the  
453 impervious surfaces connected to the stream via stormwater drainage systems. As urbanization  
454 progresses and intensifies, the proportion of connected imperviousness is expected to increase,  
455 exacerbating modifications to the flow regime (Jacobson, 2011; Burns et al., 2012). Modifying  
456 an urban channel does nothing to address this fundamental driver of flow regime and sediment  
457 supply that degrade a stream corridor over years to decades. If the fundamental driver is not  
458 addressed, then downstream actions cannot sustain themselves. This is essential so that  
459 incorporated forms are functional beyond their initial construction and propagate through to  
460 ecological functions. We propose it is possible that optimal ecosystem restoration of urban  
461 streams with demonstrable ecological benefits could be achieved if considerable effort and  
462 some thinking outside of channel-based approaches are made as suggested by Vietz et al.  
463 (2016).

464 Table 3. Average percentage increase (+) or decrease (-) of the explored hydraulic  
 465 characteristics for flows above 0.5Q<sub>bkf</sub> for each channel scenario. Values are relative to what  
 466 was predicted in the unrestored urban channel (*Urb*).

<b>Channel scenario</b>	<b>Bed disturbance (Shield stress) (%)</b>	<b>Hydraulic diversity (HMID) (%)</b>	<b>Refuge habitat (SSWH) (%)</b>
Urban channel with only width variation ( <i>Urb<sub>W</sub></i> )	- 7	+ 12	+ 4
Urban channel with only depth variation ( <i>Urb<sub>D</sub></i> )	- 12	+ 21	+ 10
Urban channel with both width and depth variation ( <i>Urb<sub>W+D</sub></i> )	- 37	+ 30	+ 21
Urban channel with both width and depth variation and meander ( <i>Urb<sub>W+D+M</sub></i> )	- 45	+ 41	+ 32

467

### 468 **4.3. Implications and opportunities for restoration of urban streams**

469 Results showed that the addition of naturalized undulations to depth, width, and centreline  
 470 position, yield more diverse hydraulics that approach natural conditions. This supports the  
 471 increasing recognition of structurally organized and harmonically coherent spatial diversity as  
 472 a central feature of aquatic systems to promote the physical template within which ecosystem  
 473 processes such as sediment transport, nutrients dynamics can occur at natural rate (Clarke et  
 474 al., 2003; Escobar-Arias & Pasternack, 2010; Lane et al., 2018). In addition, it overlaps with



475 the general consensus that the more diverse the channel the greater the ecological benefit  
476 expected (Chin & Gregory, 2009; Bernhardt & Palmer, 2011; Beagle et al., 2016). The lack of  
477 in-stream structures in this study leaves open the possibility that further improvements are  
478 possible. However, such features tend to be more prone to collapse and work best when fed  
479 and created through natural processes, whereas re-configuring the reach-scale structure is  
480 extremely different to obtain passively.

481 Key ecosystem functions associated with key stream health integrity are controlled by the  
482 mutual interplay between morphology and the hydrological regime, so channel form and flow  
483 inputs are critical (Clarke et al., 2003; Brown & Pasternack, 2014). In this regard, solving one  
484 may not necessarily address the other. While managing of other aspects of land use and channel  
485 form might be required or beneficial for an urban stream restoration, it is certain restoring  
486 altered flow regime is a prerequisite to have a chance to fully recover natural ecosystem. We  
487 propose a practicable and comprehensive stream restoration approach requires outside stream  
488 perspectives, where a broader catchment-scale management practices that addresses the source  
489 of ecosystem degradation are critically considered. Such an approach requires a consideration  
490 of flow-regime stormwater management and the application of strategies at or near the source  
491 to meet required flow regime target (Burns et al., 2014; Fletcher et al., 2014). This is in line  
492 with recently emphasized process-based restoration that expresses a broader effort of  
493 addressing the root cause of ecosystem degradation along a recovery trajectory (Beechie et al.,  
494 2010; Walsh et al., 2016). The present study presents to urban stream managers a  
495 methodological design measures that is underpinned on ecohydraulic principles. The hydraulic  
496 and geomorphic modelling approach used can be a template to understand the optimal  
497 combination of flow and morphological restoration.

498 Legacy impacts may mean separate modification of channel form and flow is required, to give  
499 managers flexibility particularly when both ecological and social values of the aquatic  
500 ecosystems are to be considered (Jacobson & Galat, 2006).

#### 501 **4.4. Uncertainties and applicability of study approach**

502 This study used an emerging technique of synthesizing channel morphology for science and  
503 engineering applications. The use of synthesis of earth landforms is a valuable element of  
504 scientific research, because it gives the opportunity to test conditions that may not be accessible  
505 in nature such that underpinning causalities can be explored (Richards, 1978; Brown et al.,  
506 2014). While this technique is promising in replicating general topographic characteristics at  
507 reach scales, it also has some limitations. This study incorporated general channel attributes  
508 scaled by generic reach-average geomorphic elements of case-study stream reaches. This could  
509 present some uncertainties to the synthesized morphologies. The chosen geomorphic attributes  
510 for modification are only some possible elements. Further, the use of a simple sinusoidal  
511 variability control function (Eq. 3) with only one term per variable means the width and depth  
512 variations were symmetrical which is presumed to be likely asymmetrical in the real stream.  
513 River Builder is capable of generating far more sophisticated undulations though harmonic  
514 combinations and blending non-trigonometric functions. More research is needed to know what  
515 functions are needed for each topographic variable.

516 In addition, the primary hydrological input into the developed hydraulic model of each tested  
517 scenario was the stage-discharge relationships, manually computed from cross-sections of the  
518 synthetic channels. While real hydrological time series of the case-study stream reaches were  
519 used in the temporal analyse of the hydraulic performance, the use of hydrological values  
520 scaled to synthetic DTMs in the hydraulic modeling present some data input uncertainties. We  
521 emphasize that these scaled values are estimates and care should be taken when using as utmost  
522 targets to inform management. Research is on-going to understand hydrological baseline

523 archetypes and their scaling in different channel archetypes (Lane et al., 2018b). Finally, this  
524 study also did not consider other key critical aspects of stream ecosystem such as water  
525 chemistry, temperature, substrate composition.

## 526 **5. Conclusions**

527 This study used a 2D ecohydraulic modeling framework to evaluate the performance of  
528 alternative channel design configurations which aimed to restore an urban-impacted stream  
529 channel. The analysis assessed the ability of the explored configurations to restore in-stream  
530 hydraulics close to their natural conditions by comparing their ecologically relevant hydraulic  
531 characteristics.

532 The results illustrated that achieving channel morphological variability in a degraded urban  
533 channel could help mitigate the influence of altered hydrological regime on the hydraulic  
534 conditions. As the variability increased, some improvement in the hydraulic conditions in terms  
535 of minimized bed mobility rate, reduced hydraulic diversity and habitat availability loss was  
536 observed. The reconfigured urban channel with bed diversity and sinuosity showed the most  
537 resilient to hydrological fluctuations offering 45% decreases in bed disturbance, 32% increases  
538 in habitat availability and 41% increases in hydraulic diversity per unit flow, compared to the  
539 unrestored channel. However, the results suggested restoring a more natural flow regime  
540 management is required, if natural hydraulic conditions are to be achieved. We argue that  
541 without the flow regime being addressed, restoring channel-based restoration attempts is likely  
542 to be hindered by the countering effect of increased magnitude, frequency and duration of  
543 disturbance flows. An integrated approach considering both reach-scale intervention and  
544 addressing catchment scale drivers of channel form is thus required.

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551

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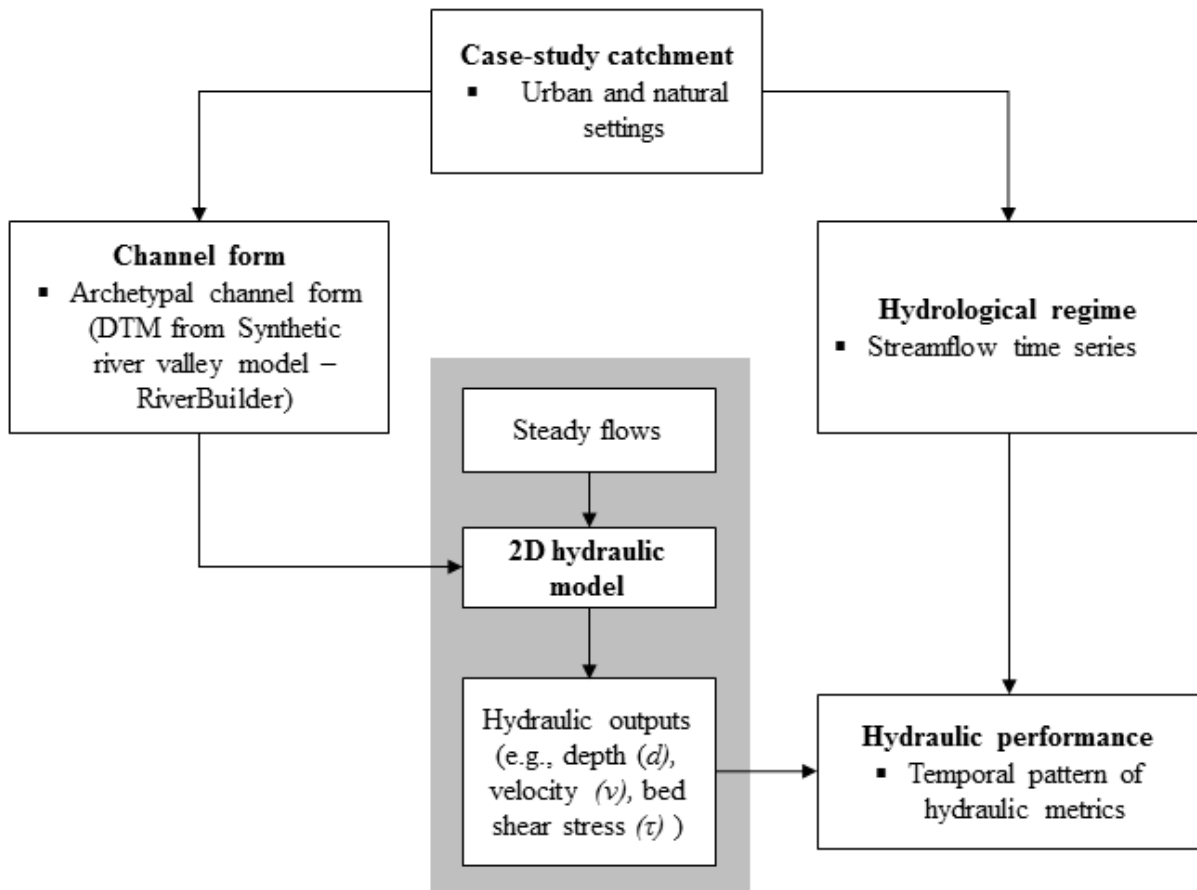
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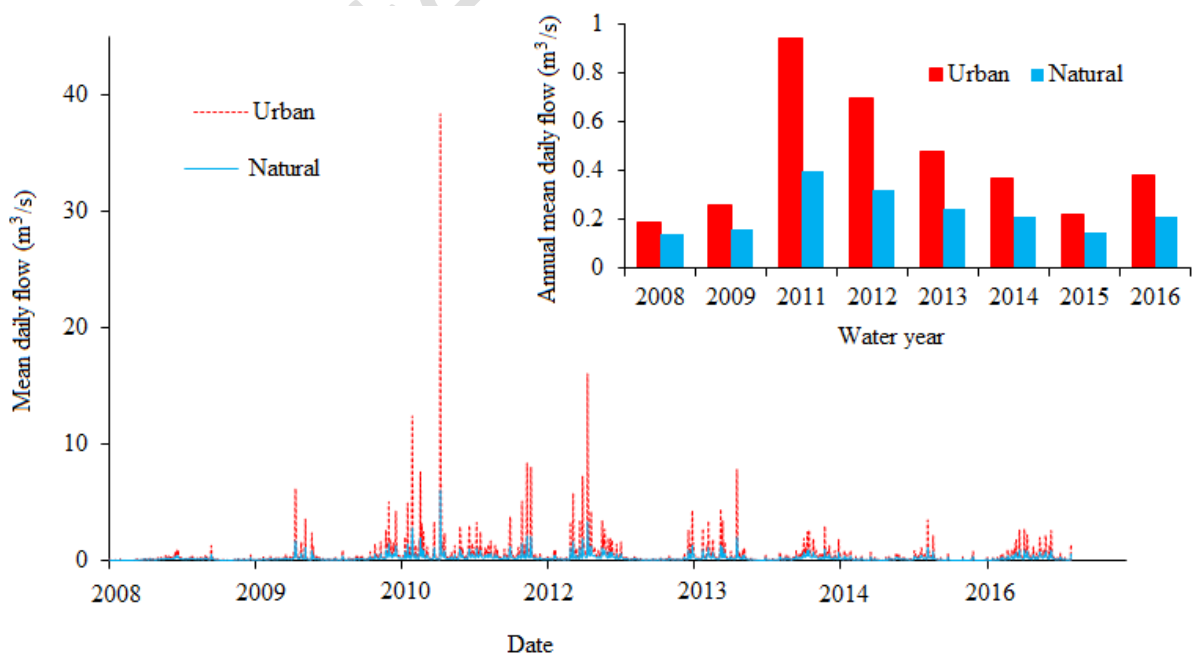
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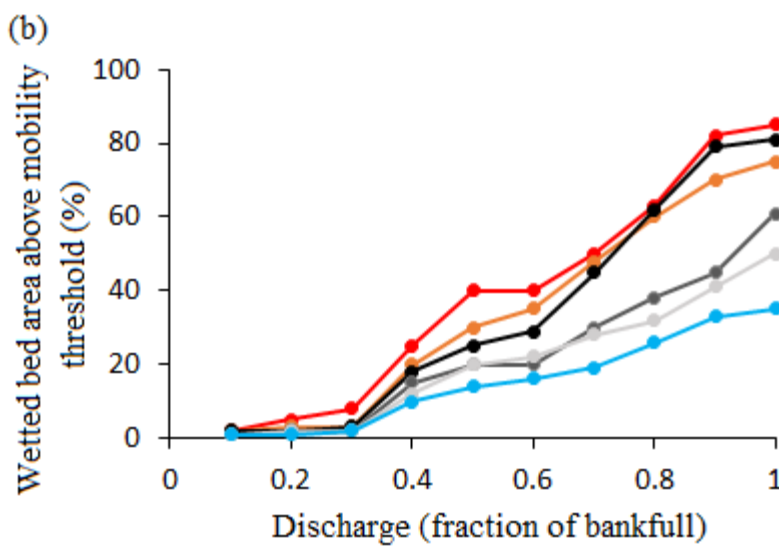
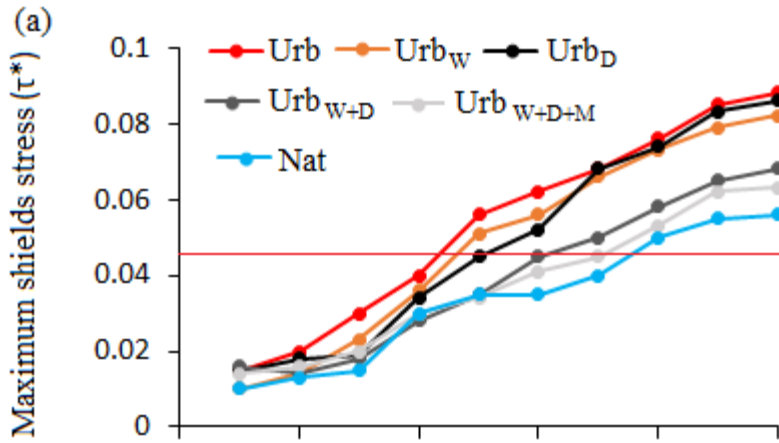
791 Figure 1. Modelling approach steps used to quantify the hydraulic impacts of each  
792 investigated channel configurations.

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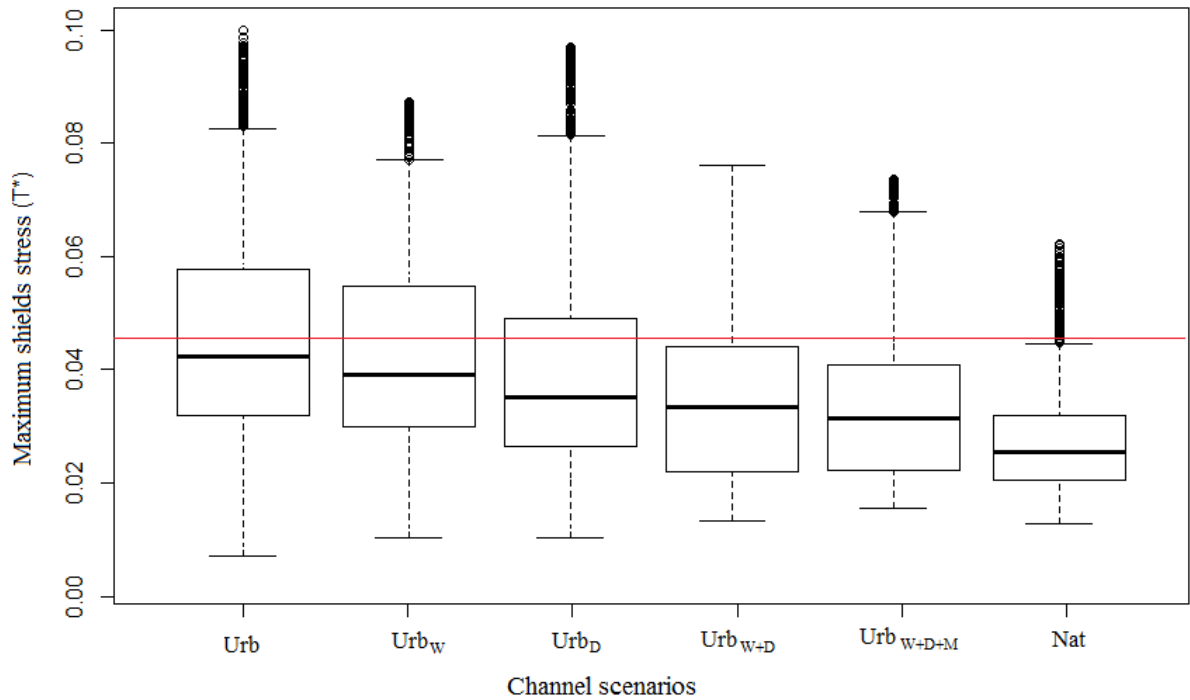
795 Figure 2. Daily flow hydrograph for the natural and urban reaches of the case-study  
796 catchment. Inset shows the annual mean daily flow for each water year.



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798 Figure 1. (a) Maximum (95<sup>th</sup> percentile) of bottom Shield stress and (b) percentage of wetted  
 799 bed area above the critical mobility threshold ( $\tau^* > \tau_c^*$ ) with discharge (as a fraction of  
 800 bankfull flow) for each channel configuration.

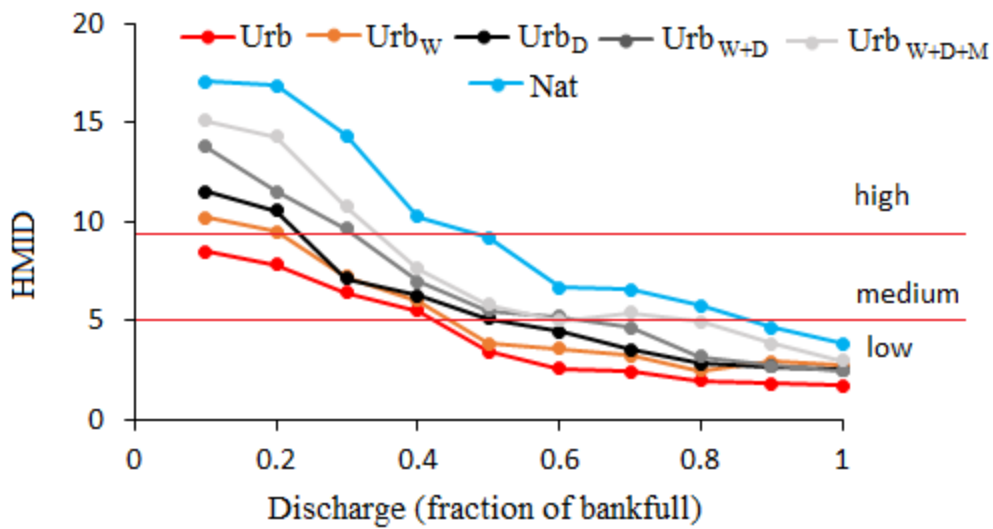
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803 Figure 2. Box and whiskers plot of the distribution of daily maximum (95<sup>th</sup> percentile) Shield  
 804 stress for each channel configuration.

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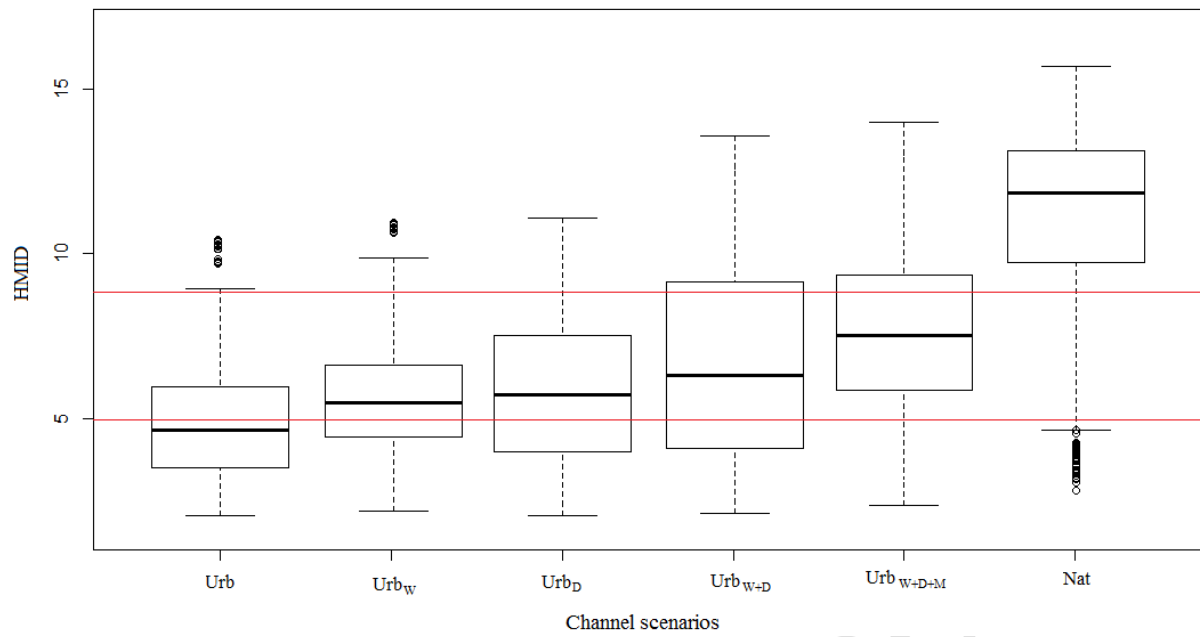


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807 Figure 3. Hydro-morphological index of diversity (HMID) with discharge (as a fraction of  
 808 bankfull flow) for each channel configuration. Red horizontal lines represent classified  
 809 threshold defined by Gostner et al. (2013).

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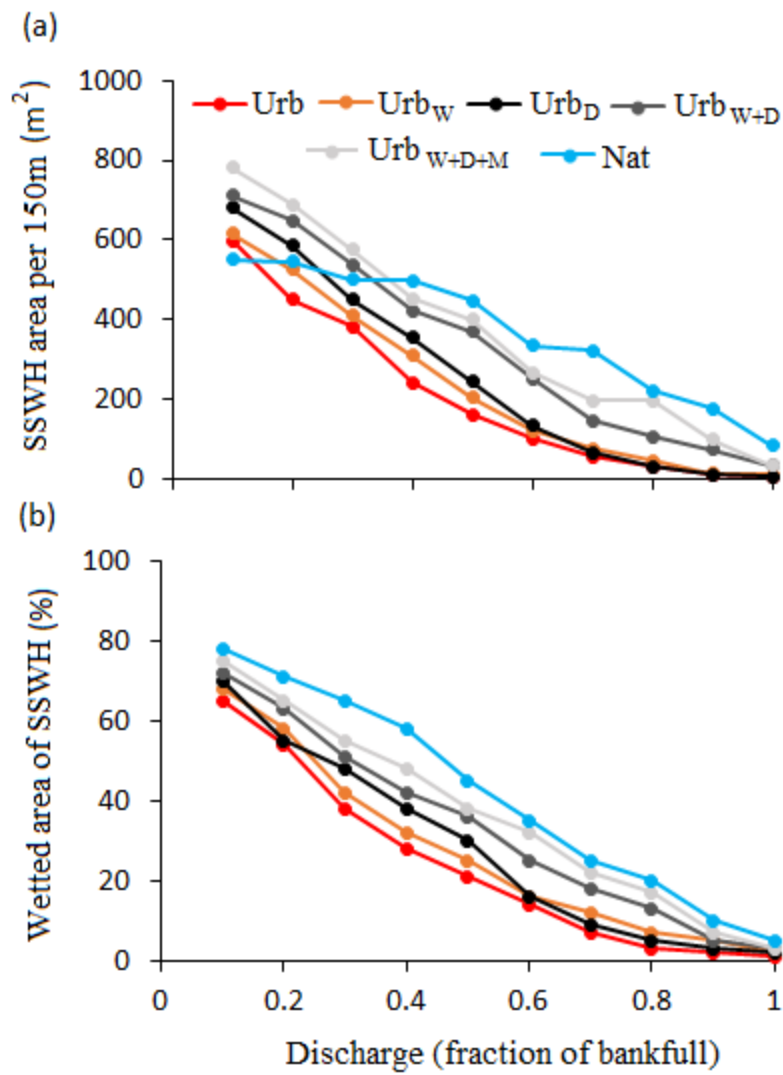


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812 Figure 4. Box and whiskers plot of the distribution of daily HMID values for each channel  
 813 configuration. Red horizontal lines represent classified threshold defined by Gostner et al.  
 814 (2013).

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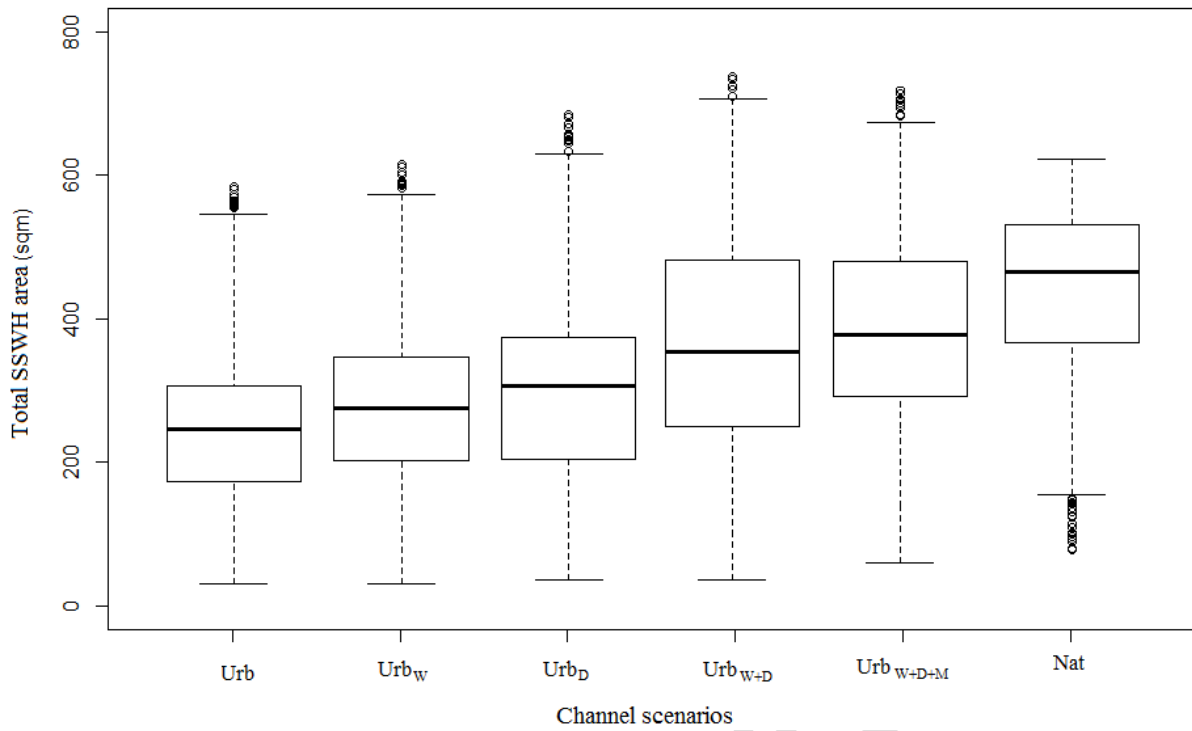
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817 Figure 5. (a) Total SSWH area per 150m<sup>2</sup> (b) percentage of total wetted bed area that is SSWH  
 818 with discharge (as a fraction of bankfull flow) for each channel configuration.

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821 Figure 6. Box and whiskers plot of the distribution of daily total SSWH area for each channel  
 822 configuration.

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