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STUDIES OF CONCRETE FOR WOLF CREEK KANSAS GAS AND ELECTRIC COMPANY POST TENSIONED REACTOR BUILDING

Final Report

by DAVID PIRTZ

Report to

Bechtel Power Corporation Gaithersburg , Maryland

April 1977

STRUCTURAL ENGINEERING LABORATORY UNIVERSITY OF CALIFORNIA BERKELEY, CALIFORNIA

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Final Report - Option 1 <u>STUDIES OF CONCRETE FOR WOLF CREEK</u>, <u>KANSAS GAS AND ELECTRIC COMPANY</u>, <u>POST TENSIONED REACTOR BUILDING</u>

1.0 SCOPE

The purpose of this test program was to establish the uniaxial creep and other mechanical and thermal properties of the proposed concrete mix design for the Wolf Creek, Kansas Gas and Electric Company, Reactor Building. The work consisted of furnishing all supervision, labor, materials, and equipment, and performance of all operation and incidentals necessary for the concrete material properties test, except as noted in Section 2.7.

The test program consisted of two options: Option I, the final test program; and, Option II, a preliminary test program, which was not performed for this reactor building.

The Option I test program required the testing of three concrete mixes.

1.1 Mix No. E-2

1.1.1 A mix with 1-1/2 in. maximum size aggregate.

1.1.2 Compressive strength of 6000 psi at 90 days.

1.1.3 Mix design supplied by Owner.

1.2 Mix No. E-1

- 1.2.1 A mix with 3/4-in. maximum size aggregate.
- 1.2.2 Compressive strength of 6000 psi at 90 days.
- 1.2.3 Mix design supplied by Owner.

1.3 Mix No. E-2-S (Special)

- 1.3.1 A mix with 1-1/2 in. maximum size aggregate.
- 1.3.2 Compressive strength less than that of the E-2 mix.
- **1.3.3** Mix design supplied by U.C.
- 1.3.4 This mix was required because the compressive strength for the 28-day E-2 mix was considerably higher than the 6000 psi compressive strength specified.

2.0 TEST PROGRAM

The Option I test program comprised the evaluation of the following properties of the concrete.

- 2.1 <u>Compressive Strength</u> determined on sealed concrete specimens, stored at 73°F, at ages of 7, 28, 90, 180, and 365 days for Class E-2 and E-2-S concrete and at ages of 28, 90, and 180 days for Class E-1 concrete.
- 2.2 <u>Modulus of Elasticity and Poisson's Ratio</u> determined on 6 by 12-in. sealed concrete specimens, stored at 73°F, at ages of 7, 28, 90, 180, and 365 days for Class E-2 and E-2-S concrete and at the age of 180 days only for Class E-1 concrete.
- 2.3 <u>Coefficient of Thermal Expansion</u> determined on two 6 by 16-in. sealed concrete specimens, stored at 73°F, at ages of 28, 180, and 365 days for Class E-2 concrete.
- 2.4 <u>Specific Heat</u> determined on two 8 by 16-in. sealed concrete specimens, stored at 73°F, at ages of 28 and 365 days for Class E-2 concrete.
- 2.5 <u>Diffusivity</u> determined on two 8-1/2 by 17-in. sealed concrete specimens, stored at 73°F, at ages of 28 and 365 days for Class E-2 concrete.
- 2.6 <u>Creep Characteristics</u> of sealed concrete specimens were determined at a sustained stress of 2100 psi, initially applied at ages of 28, 180, and 365 days for Class E-2 concrete, at age 28 days for Class E-2-S concrete, and at age 180 days for Class E-1 concrete. The autogenous strain change for the Class E-2 concrete was determined for a period of one year on sealed creep specimens that were loaded at age one year. No autogenous strain change was determined for the Class E-1 or Class E-2-S concretes. The creep tests were carried out at 73°F and 110°F for the Class E-2 concrete and at 73°F only for the Class E-1 and E-2-S concretes. Each creep test was conducted on a set of two 6 by 16-in. sealed concrete specimens.

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2.7 The following related work was not included.

2.7.1 Design of concrete mixes, except for Mix E-2-S.

- 2.7.2 Supply of portland cement, admixtures, and aggregate used for the test program.
- 2.7.3 Performance of acceptance or user tests for concrete materials.

3.0 ABBREVIATIONS

ACI	 American	Concrete Institute
ASTM	 American	Society for Testing and Materials
AISI	 American	Iron Steel Institute

4.0 CODES AND STANDARDS

Codes and standards referenced herein are listed below, together with their common abbreviations and year of adoption, as used in this Specification. Standards or codes, including the year of adoption or revision, appearing in referenced documents other than those describing test procedures or methods of sampling shall not be considered as part of this Specification unless specifically referenced below.

ASTM C 33-74	Standard Specification for Concrete Aggregates
ASTM C 39-72	Standard Method of Test for Compressive Strength of Cylindrical Concrete Specimens
ASTM C 125-74	Standard Definitions of Terms Relating to Concrete and Concrete Aggregates
ASTM C 127-73	Standard Method of Test for Specific Gravity and Absorption of Coarse Aggregate
ASTM C 128-73	Standard Method of Test for Specific Gravity and Absorption of Fine Aggregate
ASTM C 138-75	Standard Method of Test for Unit Weight, Yield, and Air Content (Gravimetric) of Concrete
ASTM C 143-74	Standard Method of Test for Slump of Portland Cement Concrete
ASTM C 150-74	Standard Specification for Portland Cement
ASTM C 192-69	Standard Method of Making and Curing Concrete Test Specimens in the Laboratory
ASTM C 231-75	Standard Method of Test for Air Content of Freshly Mixed Concrete by the Pressure Method

ASTM C 469-65	Standard Method of Test for Static Modulus of Elasticity and Poisson's Ratio of Concrete in Compression
ASTM C 566-67	Standard Method of Test for Total Moisture Content of Aggregate by Drying
ASTM C 617-73	Standard Method of Capping Cylindrical Concrete Specimens
ASTM E 4-72	Standard Method of Capping Cylindrical Concrete Specimens
ASTM E 6-73	Standard Definitions of Terms Relating to Methods of Mechanical Testing
ASTM E 12-70	Standard Definitions of Terms Relating to Density and Specific Gravity of Solids, Liquids and Gases
ASTM E 83-67	Standard Method of Verification and Classification of Extensometers

5.0 MANUFACTURE OF CONCRETE SPECIMENS

5.1 Mixing and Placing

Concrete was proportioned in accordance with the mix design and materials supplied by the Owner. The mix designs are shown in Table A.

Aggregates were prepared in accordance with ASTM C 192, Section 4.3. Bulk specific gravity and absorption were determined for the aggregates in accordance with ASTM C 127 and C 128, and are reported in Table B. Cement was stored in a steel moisture-proof container.

Mixing of concrete was in accordance with ASTM C 192, Section 5.1.2. Cement and aggregates were stored at 73°±3°F for at least two days before mixing to assure a uniform temperature of these materials. Slump was measured in accordance with ASTM Method C 143. Entrained air content was measured in accordance with ASTM Method C 231. Unit weight and yield were determined in accordance with ASTM C 138. Specimens were made and consolidated in accordance with ASTM C 192, Sections 5.3 and 5.4.3, respectively. Accurate records of the mix proportions, moisture content of aggregate, air content, unit weight, and yield were retained.

The specimens for the creep and thermal coefficient of expansion were cast in 6.000 inches (within a tolerance of -.002 inches) by 18 inches machined split cast iron molds. Prior to casting, one Carlson 8-inch strain gage, properly calibrated, was centered on the axis of the

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cast iron mold. The lead wire from the strain gage was brought out through a hole drilled in the center of a 2-inch thick plate placed at the bottom of the mold and sealed by means of an "O" ring. The final specimen length was 16 inches.

A 1/8-in. by 8-in. metal rod was placed diametrically across the top of this mold to serve as a support for a wire which held the meter in an axial position during casting. After casting, the wire was cut-off, the rod removed, and the top of the cast iron mold sealed with Saran wrap.

The creep and thermal expansion specimens were allowed to set for five hours after casting to allow bleeding water to be reabsorbed prior to capping. Then, a conical-shaped layer of mortar made from the original mix was formed on the top of each cylinder. The 1-1/2 in. thick steel top-plates were then worked back and forth into position until, the mortar appeared to be spread uniformly between the plate and the specimen. A leveling plate was used to assure that each top-plate was normal to the axis of the specimen. The creep and thermal expansion specimens were than moved to the 73°F, 50 percent relative humidity room.

The split cast iron molds were stripped from the creep and thermal expansion specimens at the age of one day. Within three minutes after removal of the cast iron mold, a 1/16-in. thick butyl rubber sheet was wrapped and bonded to the top and bottom steel plates with rubber cement. A 3-in. wide lap splice was used to join the butyl rubber sheet. Large hose clamps were placed over the butyl rubber and the end steel plates to assure that the specimens would be internally sealed. The specimens tested at 73°F remained in the 73°F, 50 percent relative humidity room. The specimens tested at 110°F were moved to a 90°F room at the age of eight days. At the age of 16 days, they were moved to the 110°F room where they remained until the end of the test.

Compressive strength specimens were cast in 6 by 12-in. sheetmetal cans. The lid and all joints were sealed with silicon rubber to internally seal the specimens. All sealed compressive strength specimens remained in the 100 percent relative humidity room until just prior to testing, at which time they were stripped, capped, and covered with Saran wrap to ensure water retention throughout the test period.

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Modulus of elasticity and Poisson's ratio were determined on the compressive strength cylinders.

Specimens for thermal diffusivity tests were cast in 8-1/2 by 17 by 0.020-in. thick steel cans. They were cast solid, except for a 3/8-in. diameter by 8-1/2 in. deep thermometer well centered on the axis of the specimen. After casting, lids were placed on the specimens and the cans were sealed with silicon rubber prior to being moved to the 73°F, 100 percent relative humidity room. The external metal container was left on the cylinders throughout the duration of the test.

Specimens for the specific heat tests were cast in 8 by 16 by 0.020-in. thick copper cans. They were cast solid, except for a 1-5/8 in. 0.D. by 1-1/2 in. I.D. brass tube centered on the axis for the full length of the specimen. After casting, lids were placed on the specimens and the cans were sealed with silicon rubber prior to being moved to the 73°F, 100 percent relative humidity room. The external metal container remained on the cylinders throughout the duration of the test.

5.2 Curing Procedure

After each specimen was consolidated and finishing of the top surface was completed, it was placed in a room under the environmental conditions specified herein for the required test.

6.0 TEST RESULTS

6.1 Mix Design Data

The mix design and data for the concrete mixes used in casting the specimens are shown in Tables C to H. In Tables C to F, the mix designs were computed using absorptions of the aggregate supplied by the Owner. In Tables F to H, the mix designs were computed using absorption of the aggregate determined at Berkeley. In Tables C to H, the weight of cement, water, sand, 3/4-in. and 1-1/2-in. aggregates per cubic yard of concrete were computed using the measured unit weight of the concrete and the batch weights of each material. [Weight of each material, pcy = (Batch weight of each material, lbs.) x (Unit weight of concrete, pcy) \div (Total batch weight, lbs.)]

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6.2 Compressive Strength and Elastic Properties

Compressive strengths were determined at the ages of 7, 28, 90, 180, and 365 days for Class E-2 and Class E-2-S concretes, and at ages 7, 28, 90, and 180 days for Class E-1 concrete. The average diameter of each specimen was between 5.96 in. and 5.98 in. The ends of the cylinders, to which loads were applied, were plane square end surfaces at right angles to the axis of the specimen and met the planeness requirements of Section 1.2 of ASTM Method C 617. Each specimen was checked for planeness. Testing procedures were in accordance with ASTM C 469, Sections 4.3 through 4.7, inclusive. The testing machine and compressometer used comply with ASTM C 469, Section 2. Each strength determination represents the average obtained from three 6 by 12-in. cylinders. The same three 6 by 12-in. concrete cylinders were used in the determination of compressive strength, modulus of elasticity (E), and Poisson's ratio (μ). The modulus of elasticity and Poisson's ratio were determined by use of an XYY recorder employing differential transformers. This arrangement produces a continuous plot of stress versus longitudinal strain and lateral strain versus longitudinal strain from which both the modulus of elasticity (E) and Poisson's ratio (μ) were computed. The loading rate used was 60,000 lbs. per minute, which is equivalent to 35 psi per second for a 6-in. diameter specimen. Compressive strength, modulus of elasticity, and Poisson's ratio for sealed concrete specimens stored at 73°F and 100 percent relative humidity are shown in Table I.

6.3 Thermal Diffusivity

The average 28 and 365-day thermal diffusivities for Class E-2 concrete, as determined on two 8-1/2 in. diameter by 17-in. long concrete cylinders, were 0.031 and 0.032 ft²/hr, respectively, and are shown in Table I. The hot water bath and cold water bath were approximately 120°F and 40°F, respectively, for the thermal diffusivity tests.

Thermal diffusivity was determined by the cooling test described in "Thermal Properties of Concrete," Bulletin 1, United States Bureau of Reclamation, Boulder Canyon Project, Final Reports, 1940, pp. 66-86 and pp. 133-143.

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6.4 Specific Heat

The average 28 and 365-day specific heat for Class E-2 concrete, as determined on two 8-in. diameter by 16-in. long concrete cylinders, were 0.262 and 0.252 Btu/lb. x $^{\circ}$ F, respectively, and are shown in Table I.

Specific heat was determined with an adiabatic calorimeter designed to measure the amount of heat required to raise the temperature of a cylindrical test specimen. The calorimeter consisted essentially of two double-walled containers, one within the other, with provision for the measurement of heat input and temperature rise. The equipment and method used were essentially the same as described in "Thermal Properties of Concrete," Bulletin 1, U.S. Bureau of Reclamation, Boulder Canyon Project, Final Report, 1946, pp. 26 and 27 and pp. 112 to 117.

6.5 Thermal Coefficient of Expansion

The two sealed 6-in. by 16-in. thermal coefficient of expansion specimens containing Class E-2 concrete were measured for length changes by means of a Carlson strain meter at successive temperatures of 73°F, 40°F, 100°F, 73°F, 100°F, and 73°F. Specimens were left for at least 24 hours at each temperature before strain readings were taken. At the end of the cycling period, the specimens were stored at 73°F. The average linear thermal expansion for the two specimens at ages of 28, 180, and 365 days were '3.3, 3.4, and 3.5 micro-strain per 1°F temperature change, respectively.

6.6 Sustained Modulus of Elastic, Creep, and Autogenous Strains

Creep characteristics for the concrete were determined on sealed 6-in. by 16-in. cylinders with centrally embedded Carlson strain meters. Sixteen specimens were initially loaded at different ages and temperatures as shown in the table below.

Class of Concrete	Temp. of Specimen			ecimens at Age, days
		28	180	365
E-2	73°F	2	2	2
E-2	110°F	2	2	2
E-1	73°F		2	qaa
E-2-S	73°F	2	-	630-

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The autogenous strains for the 28 and 180-day loaded creep specimens with Class E-2 concrete were determined from strain data before loading on the specimens with Class E-2 concrete, which were loaded at 365 days. No specimens were cast to determine autogenous strains for the Class E-1 concrete so the strains were assumed to be zero. The autogenous strains for the 28-day loaded creep specimens with Class E-2-S concrete were determined from data before loading on the specimens with Class E-2 concrete, which were loaded at 365 days.

The loading frames used are capable of applying and maintaining a stress level of 2100 psi to all loaded creep specimens despite any change in the dimension of the specimen. Each frame is capable of accepting two specimens in tandem (lengthwise) for simultaneous loading. The frame consists of two header plates (thickness of 1 inch) connected by three 1-1/2 in. (AISI C 1215) steel rods. Care was taken to prevent eccentric loading on all specimens. The hydraulic load-maintaining element consisted of accumulators, regulators, indicator gages, and a high pressure pump which is used to maintain the load on each frame. Pressure gages provide a means for measuring the load to the nearest 2 percent of the total applied stress.

For applying the initial stress of 2100 psi, a manual hand pump was used to apply the stress at a uniform rate of 35±5 psi per second. At this rate the total stress was applied in 60 seconds. Each loaded creep specimen's strain gage was read at: -60 seconds (no load applied); zero time (full load applied); one minute; 10 minutes; two hours; eight hours; 24 hours; daily for one week; weekly for one month; and, twice monthly thereafter.

Sustained modulus of elasticity, creep characteristics, and autogenous strains for sealed concrete specimens are shown in Tables J to Q.

Table J	* ****	Class E-2 concrete stored at 73°F and stressed for 339 days starting at age 28 days.
Table K	*****	Class E-2 concrete stored at 110°F and stressed for 339 days starting at age 28 days.
Table L	-	Class E-2 concrete stored at 73°F and stressed for 217 days starting at age 180 days.
Table M	way	Class E-2 concrete stored at 110°F and stressed for 217 days starting at age 180 days.

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- Table N Class E-2 concrete stored at 73°F and stressed for 60 days starting at age 365 days.
- Table 0 Class E-2 concrete stored at 110°F and stressed for 60 days starting at age 365 days.
- Table P Class E-1 concrete stored at 73°F and stressed for 166 days starting at age 180 days.
- Table Q Class E-2-S concrete stored at 73°F and stressed for 318 days starting at age 28 days.

In the above tables, a minus time under stress indicates a time prior to full load and zero time under stress indicates the time when full load was reached. The sustained modulus of elasticity was computed by dividing the applied stress of 2100 psi by the sum of elastic, creep, and autogenous strains. The autogenous strain values shown are based on a zero value at time of full load.

Elastic plus creep plus autogenous strains, creep plus autogenous strains, and creep strains are all shown plotted versus log of time plus one day for the average of two unsealed concrete specimens in Figures 1 to 8.

Class E-2 concrete stressed at age 28 days

Figure 1 -

and stored at 73°F. Figure 2 -Class E-2 concrete stressed at age 28 days and stored at 110°F. Class E-2 concrete stressed at age 180 days Figure 3 and stored at 73°F. Figure 4 - Class E-2 concrete stressed at age 180 days and stored at 110°F. Figure 5 -Class E-2 concrete stressed at age 365 days and stored at 73°F. Figure 6 - Class E-2 concrete stressed at age 365 days and stored at 110°F. Figure 7 -Class E-1 concrete stressed at age 180 days and stored at 73°F. Figure 8 -Class E-2-S concrete stressed at age 28 days and stored at 73°F. Elastic plus creep plus autogenous strains are plotted versus time for the average of two sealed concrete specimens in Figures 9 and 10. Figure 9 - Class E-2 concrete stressed at ages of 28, 180, and 365 days and stored at 73°F. Figure 10 - Class E-2 concrete stressed at ages of 28, 180, and 365 days and stored at 110°F.

The complete computer calculations for determining the strains due to loading the sealed concrete specimens are shown in Tables 1A/1B to 8A/8B.

Tables 1A & 1B - Class E-2 concrete stressed at age 28 days and stored at 73°F. Tables 2A & 2B - Class E-2 concrete stressed at age 28 days and stored at 110°F. Tables 3A & 3B - Class E-2 concrete stressed at age 180 days and stored at 73°F. Tables 4A & 4B - Class E-2 concrete stressed at age 180 days and stored at 110°F. Tables 5A & 5B - Class E-2 concrete stressed at age 365 days and stored at 73°F. Tables 6A & 6B - Class E-2 concrete stressed at age 365 days and stored at 110°F. Tables 7A & 7B - Class E-1 concrete stressed at age 180 days and stored at 73°F. Tables 8A & 8B - Class E-2-S concrete stressed at age 28 days and stored at 73°F.

The complete computer calculations for determining the autogenous strains are shown in Tables 9A/9B and 10A/10B.

Tables 9A & 9B - Class E-2 concrete stored at 73° F. Tables 10A & 10B - Class E-2 concrete stored at 110°

7.0 COMMENTS

7.1 For the first mix, Class E-2 concrete, using the mix design supplied by Kansas Gas and Electric Company, aggregate absorptions obtained at Berkeley, a pan-type mixer slump of 5-1/2 in., and an air content of 14 percent were measured. Because the slump and air content were greater than specified, Mr. Daye of the Bechtel Corporation was contacted by phone on February 4, 1976, prior to casting any specimens. A decision was then made to use the aggregate absorptions as given by Mr. Daye for all further mixes since this reduced the calculated free water and, therefore, the slump.

7.2 A Lancaster counter current batch pan mixer, Type 30 DH, was used for all of the mixes. Use of this mixer may have accounted for the decrease in air-entraining agent used to obtain the required 3 to 6 percent air. Also, due to better mixing, this pan mixer may have accounted for some of the increase in compressive strength for mixes made at Berkeley.

7.3 Bechtel specifications for slump are given for field conditions measured at point and time of placement. Clarification was needed as to what procedure to follow in the laboratory to measure the desired slump. For the mixes made at Berkeley, slump was measured three minutes and eight minutes after end of mixing.

7.4 All work was performed in accordance with the "Quality Assurance Program" submitted prior to the start of testing.

TABLE A

WOLF CREEK POST TENSIONED REACTOR BUILDING

Materia	<u>1</u>	Source			
Cement:		Ashgrove Type II			
Sand:		Christie Quarry, Lomont, Kansas			
3/4 in.	Aggregate:	Christie Quarry, Lomont, Kansas			
1-1/2 i	n. Aggregate:	Christie Quarry, Lomont, Kansas			
WRA Adm	ixture:	Master Builders, Pozzolith 300N			
AEA Admixture:		Master Builders, MV-BR			
<u>Specifi</u>	cations				
Compres	sive Strength:	6,000 psi at 90 days			
Slump:	Working limit at point of placement – Inadvertency Margin – Rejection Limit –	3 inches - Mix E-1 2-1/2 inches - Mix E-2 2 inches - Mix E-1 and Mix E-2 5 inches - Mix E-1 4-1/2 inches - Mix E-2			
Air:		3 to 6 percent			
Tempera	ture:	73°F ± 3°F			
Weights	(S.S.D.) for One Cubic Yard	of Concrete			

Weights (S.S.D.) for One Cubic Yard of Concrete
(as per letter from Mr. Thomas F. Regan, Kansas Gas & Electric Company,
dated September 24, 1975)

Mix No.	E-1	E-2
Maximum Size Aggregate:	3/4 in.	1-1/2 in.
Cement, lbs.	750	740
Water, lbs.	307	290
Sand, 1bs.	1400	1271
3/4 in. Aggregate, 1bs.	1480	820
1-1/2 in. Aggregate, lbs.	uain bite	820
WRA, fl. oz.	37.5	37.0
AEA, fl. oz.	6.6	6.9

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TABLE B

WOLF CREEK POST TENSIONED REACTOR BUILDING

Bulk Specific Gravity (Saturated Surface Dry) Absorption Capacity, Aggregate percent Berkeley Owner Sand 2.62 2.6 1.9 3/4 in. Aggregate 2.62 2.2 1.8 1-1/2 in. Aggregate 2.61 1.9 1.4

Bulk Specific Gravity and Absorption Capacity

Note: Aggregates not initially oven dried in the determination of these values.

TABLE C

CASTING DATA FOR MIX E-2

Mix designs are computed using absorption of aggregates supplied by Owner.

Date		February 4, 1976				March 26, 1976			
Specimens Cast	2 -	<pre>12 - 6x16-in. creep specimens 2 - 6x16-in. thermal expan- sion cylinders 14 - 6x12-in. cylinders 1 - 8-1/2x17 in. diffusivity cylinder 1 - 8x16-in. specific heat cylinder</pre>		9 - 6xl2-in. cylinders 1 - 8-1/2xl7-in. diffu- sivity cylinder 1 - 8xl6-in. specific heat cylinder					
Batch No.	3	4	5	6	11	12	Avg.		
Batch Size, cu. ft.							3		
Cement, pcy	737	742	738	741	737	743	740		
Water, pcy	289	291	289	290	289	291	290		
Sand, pcy S.S.D.	1266	1274	1267	1272	1266	1276	1271		
3/4 in. Aggregate, pcy S.S.D	817	822	817	821	817	823	819		
1-1/2 in. Aggregate, pcy S.S.D.	817	822	817	821	817	823	819		
AEA, oz./cu. yd.	6.9	6.9	6.9	6.9	6.9	7.0	6.9		
WRA, oz./cu. yd.	37.0	37.0	37.0	37.0	37.0	37.1	37.0		
Unit Wt., pcf	145.4	146.4	145.5	146.1	145.4	146.5	145.9		
lst Slump, in.	3-1/2	3-7/4	3-3/4	4	4-3/4	3	3-3/4		
2nd Slump, in.	2-1/ 2	2	2-1/2	2-3/4	3-1/4	2-1/4	2-1/2		
Air, % by volume	4.2	4.2	4.7	4.7	4.8	3.7	4.4		
Temp., °F ^(b)	68	68	68	69	72	72	69		
W/C Ratio by wt.	0.392	0.392	0.392	0.391	0.392	0.392	0.392		

First slump was taken three minutes after the end of mixing.
 Second slump was taken eight minutes after the end of mixing.

TABLE D

CASTING DATA FOR MIX E-2-S

Mix designs are computed using absorption of aggregates supplied by Owner.

Date	March 26, 1976				
Specimens Cast	2 - 6xl6-in. creep specime 15 - 6xl2-in. cylinders				
Batch No.	13	13 14			
Batch Size, cu. ft.	1.8	1.8			
Cement, pcy	591	593	592		
Water, pcy	270	270	270		
Sand, pcy S.S.D.	1387	1391	1389		
3/4 in. Aggregate, pcy S.S.D.	832	834	833		
1-1/2 in. Aggregate, pcy S.S.D.	832	834	833		
AEA, oz./cu. yd.	1.9	1.9	1.9		
WRA, oz./cu. yd.	30.2	30.2	30.2		
Unit Wt., pcf	144.9	145.3	145.1		
lst Slump, in.	3-3/4	4	3-3/4		
2nd Slump, in.	2-3/4	2-1/2	2-5/8		
Air, % by volume	4.3	4.4	4.4		
Temp., °F ^(b)	71	72	72		
W/C Ratio by wt.	0.457	0.455	0.456		

(1) First slump was taken three minutes after the end of mixing.(2) Second slump was taken eight minutes after the end of mixing.

TABLE E

CASTING DATA FOR MIX E-1

Mix designs are computed using absorption of aggregates supplied by Owner.

Date	March 17, 1976	March 26, 1976		76	
Specimens Cast	9 - 6x12-in. cylinders	2 - 6x16-in. creep 9 - 6x12-in. cylind			
Batch No.	8	9	10	Avg.	
Batch Size, cu. ft.	1.8	1.8	1.8		
Cement, pcy	747	746	742	745	
Water, pcy	305	305	304	305	
Sand, pcy S.S.D	1393	1392	1385	1390	
3/4-in. Aggregate, pcy S.S.D.	1473	1472	1464	1470	
1-1/2 in. Aggre- gate, pcy S.S.D.			agas data man		
AEA, oz./cu. yd.	2.2	2.2	2.2	2.2	
WRA, oz/cu. yd.	37.5	37.5	37.5	37.5	
Unit Wt., pcf	145.3	145.0	144.2	144.8	
lst Slump, in.	2-3/4	3-1/4	3-1/2	3-1/8	
2nd Slump, in.	2	2-1/2	2-1/2	2-1/3	
Air, % by volume	4.6	4.9	4.9	4.8	
Temp., ^o F ^(b)	73	71	72	72	
W/C Ratio by wt.	0.408	0.409	0.410	0.409	

First slump was taken three minutes after the end of mixing. (1) (2)

Second slump was taken eight minutes after the end of mixing.

TABLE F

CASTING DATA FOR MIX E-2

Mix designs are computed using absorption of aggregates determined at Berkeley.

Date		February 4, 1976				March 26, 1976		
Specimens Cast	 12 - 6x16-in. creep specimens 2 - 6x16-in. thermal expansion cylinders 14 - 6x12-in. cylinders 1 - 8-1/2x17-in. diffusivity cylinder 1 - 8x16-in. specific heat cylinder 			9 - 6x12-in. cylinders 1 - 8-1/2x17-in. diffu- sivity cylinder 1 - 8x16-in. specific heat cylinder				
Batch No.	3	4	5	6	11	12	Avg.	
Batch Size, cu. ft.								
Cement, pcy	737	742	738	741	737	743	740	
Water, pcy	272	274	273	274	272	274	273	
Sand, pcy S.S.D.	1275	1284	1276	1282	1275	1285	1279	
3/4-in. Aggregate, pcy S.S.D.	820	826	821	824	820	826	823	
1-1/2-in. Aggre- gate, pcy S.S.D.	820	826	821	825	820	826	823	
AEA, oz./cu. yd.	6.9	6.9	6.9	6.9	6.9	7.0	6.9	
WRA, oz./cu. yd.	37.0	37.0	37.0	37.0	37.0	37.1	37.0	
Unit Wt., pcf	145.4	146.4	145.5	146.1	145.4	146.5	145.9	
lst Slump, in.	3-1/2	3-1/4	3-3/4	4	4-3/4	3	3-3/4	
2nd Slump, in.	.2-1/2	2	2-1/2	2-3/4	3-1/4	2-1/4	2-1/2	
Air, % by volume	4.2	4.2	4.7	4.7	4.8	3.7	4.4	
Temp., °F ^(b)	68	68	68	69	72	72	69	
W/C Ratio by wt.	0.369	0.369	0.370	0.370	0.369	0.369	0.369	

(1) First slump was taken three minutes after the end of mixing.(2) Second slump was taken eight minutes after the end of mixing.

TABLE G

CASTING DATA FOR MIX E-2-S

Mix designs are computed using absorption of aggregates determined at Berkeley.

Date	Mar	ch 26, 1976	
Specimens Cast		in. creep s in. cylinde	
Batch No.	13	14	Avg.
Batch Size, cu. ft.	1.8	1.8	
Cement, pcy	591	593	592
Water, pcy	254	254	254
Sand, pcy S.S.D.	1397	1401	1399
3/4-in. Aggregate, pcy S.S.D.	.835	837	836
1-1/2-in. Aggregate, pcy S.S.D.	835	837	836
AEA, oz./cu. yd.	1.9	1.9	1.9
WRA, oz./cu. yd.	30.2	30.2	30.2
Unit Wt., pcf	144.9	145.3	145.1
lst Slump, in.	3-3/4	4	3-7/8
2nd Slump, in.	2-3/4	2-1/2	2-3/8
Air, % by volume	4.3	4.4	4.4
Temp., °F ^(b)	71	72	72
W/C Ratio by wt.	0.430	0.428	0.429

(1) First slump was taken three minutes after the end of mixing.(2) Second slump was taken eight minutes after the end of mixing.

TABLE H

CASTING DATA FOR MIX E-1

Mix designs are computed using absorption of aggregates determined at Berkeley.

Date	March 17, 1976		March 26	, 1976
Specimens Cast	9 - 6x12-in. cylinders	2 - 9 -	6x16-in. cre 6x12-in. cyl	ep specimens inders
Batch	8	9	10	Avg.
Batch Size, cu. ft.				
Cement, pcy	747	746	742	745
Water, pcy	287	287	286	287
Sand, pcy S.S.D.	1403	1402	1395	1400
3/4-in. Aggregate, pcy. S.S.D.	1478	1477	1469	1475
1-1/2-in. Aggregate, pcy S.S.D.			500 MIN 193	
AEA, oz./cu. yd.	2.2	2.2	2.2	2.2
WRA, oz./cu. yd.	37.5	37.5	37.5	37.5
Unit Wt., pcf	145.3	145.0	144.2	144.8
lst Slump, in.	2-3/4	3-1/4	3-1/2	3-1/8
2nd Slump, in.	2	2-1/2	2-1/2	2-1/3
Air, % by volume	4.6	4.9	4.9	4.8
Temp., ^o F ^(b)	73	71	72	72
W/C Ratio by wt.	0.384	0.385	0.385	0.385

First slump was taken three minutes after the end of mixing. (1) (2)

Second slump was taken eight minutes after the end of mixing.

TABLE I

MECHANICAL AND THERMAL PROPERTIES

Property	Age,		Concreie, cla	ass
rioperty	days	E-1 3/4"	É-2 1-1/2"	E-2-S 1-1/2"
Compressive Strength, psi (a)	7 28 90 180 365	6820 8050 9110(c) 8900(d) 9210 xxxx	6625 8200 9030(e) 8740(f) 9340 9990	5540 6770 7580 xxxx 7750 7940
Poisson's Ratio (a)	7 28 90 180 365	xxxx xxxx xxxx 0.23 xxxx	0.23 0.23 0.24 0.24 0.25	xxxx 0.23 xxxx xxxx xxxx xxxx
Modulus of Elasticity, psi (psi x 10 ⁶) (a)	7 28 90 180 365	xxxx xxxx xxxx 4.7 xxxx	4.5 4.7 4.8 5.1 5.1	xxxx 4.6 xxxx xxxx xxxx xxxx xxxx
Linear Thermal Expan- sion, micro-strain/°F (b)	28 180 365	xxxx xxxx xxxx	3.3 3.4 3.5	xxxx xxxx xxxx
Diffusivity, ft²/hr	28 365	xxxx xxxx	0.031 0.032	xxxx xxxx
Specific Heat, Btu/lb. °F	28 365	xxxx xxxx	0.262 0.242	xxxx xxxx

Notes: All tests done on sealed specimens.

xxxx - specimens not tested at these ages

- (a) Average of three specimens.
- (b) Average of two specimens.
- (c) Specimens from Mix No. 8 cast on 3/17/76.
- (d) Specimens from Mix No. 9 and Mix No. 10 cast on 3/26/76.
- (e) Specimens from Mix Nos. 3, 4, & 5 cast on 2/4/76.
- (f) Specimens from Mix No. 11 and 12 cast on 3/26/76.

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7.1 2.23 2.23 1.940 7.0 2.23 1.940 7.0 2.22 1.946 7.6 2.22 1.966 7.6 2.22 1.966	400 401 401 401 401 401 401	504	

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257:1 2:05 -1020 -572 -21 -503 -1027 317:1 2:03 -1027 -572 -37 -523 -27 317:1 2:02 -1027 -572 -523 -27 325:5 2:01 -1040 -572 -523 -27 336:5 2:01 -1047 -579 -36 -525 338:5 2:01 -1047 -579 -36 -540 -275 338:5 2:01 -1047 -579 -36 -540 -275 (A) SESTAINED WEDULIS OF ELASTIC -579 -36 -540 -267 (A) SFECIFIC CREEP & AND AUTGOFNOLS STRAINS. -570 -540 -267 (A) SFECIFIC CREEP & CREEP STPAIN DIVIDED BY 200, PSI 0.00, PSI	246.		146 52 246 52	~~ (1110.00	242	and a second	
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TABLE 6A

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TABLE 7A

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TABLE 7B

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TABLE 8B

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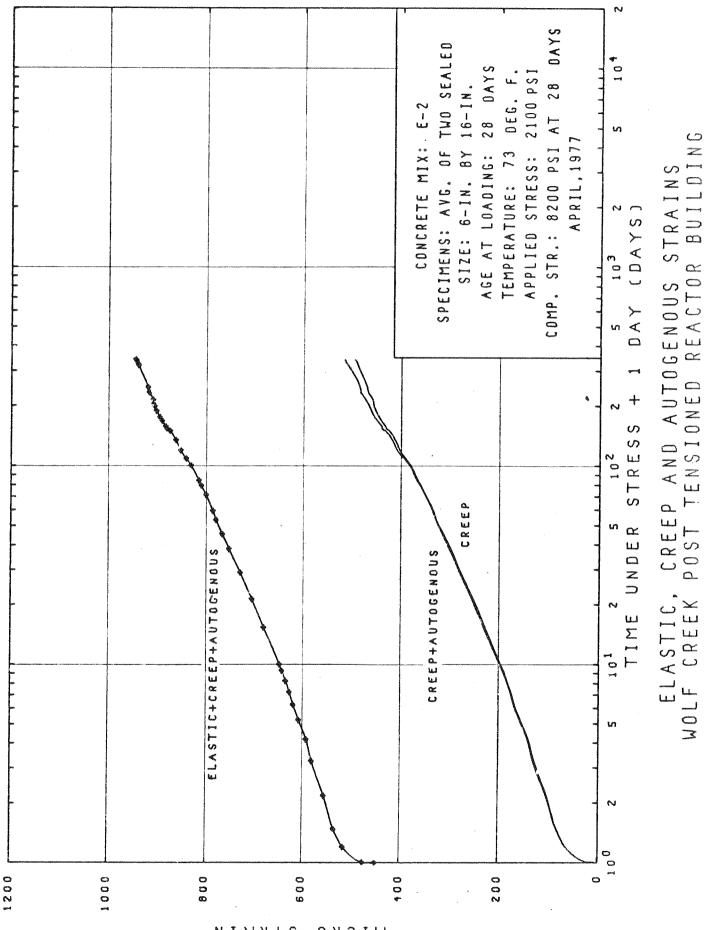
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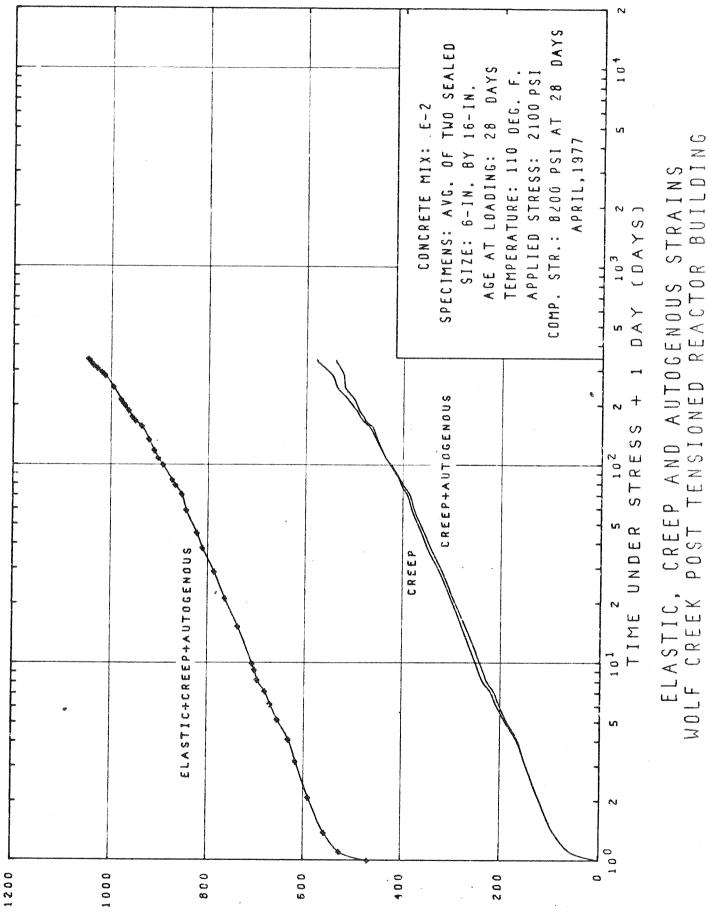
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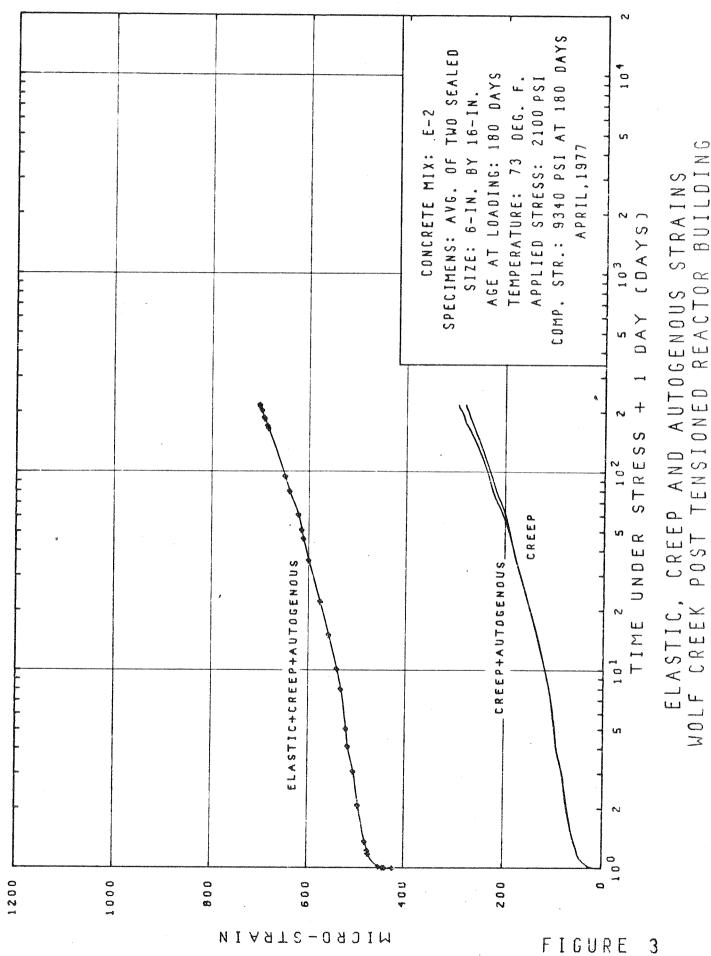
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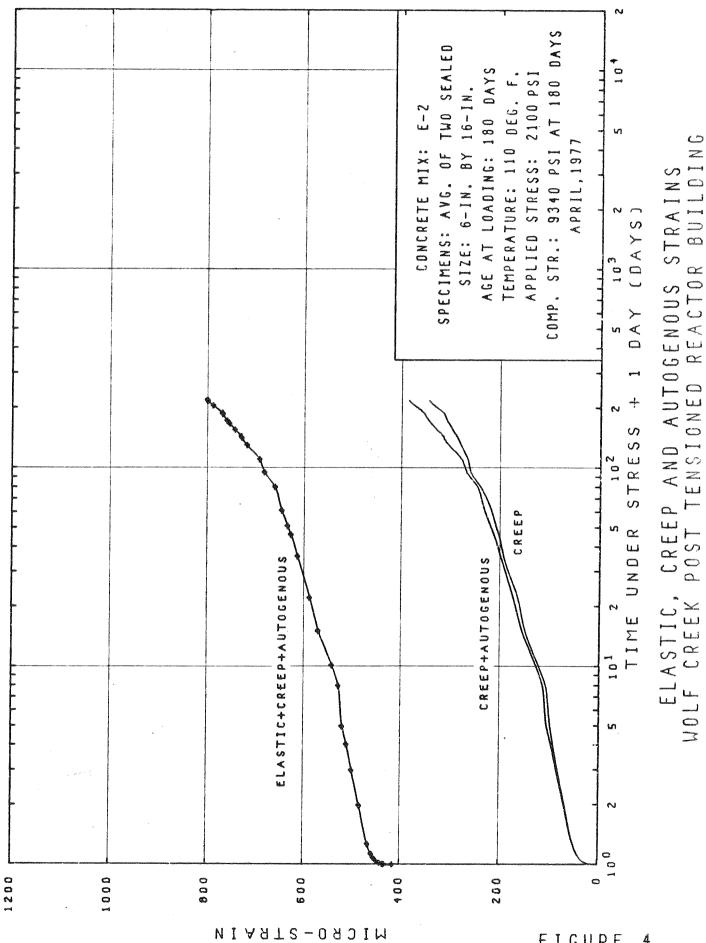
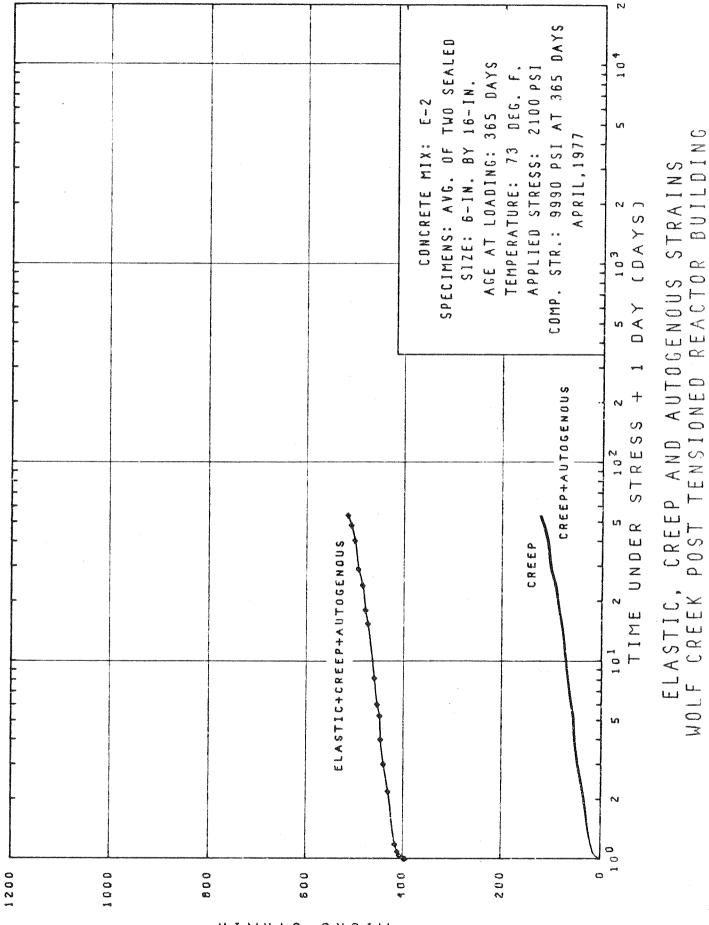
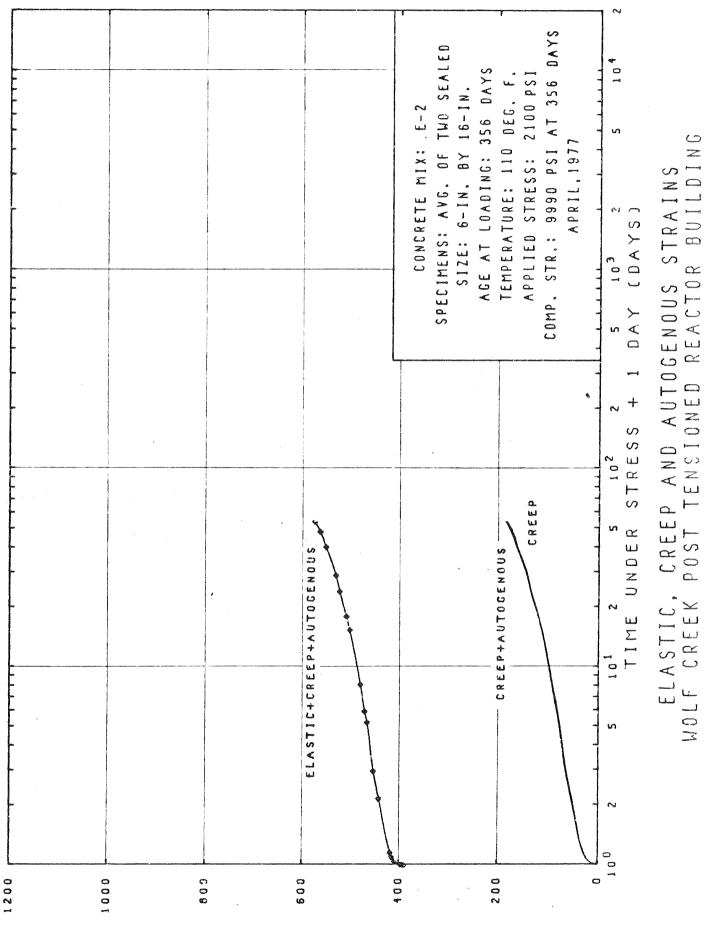
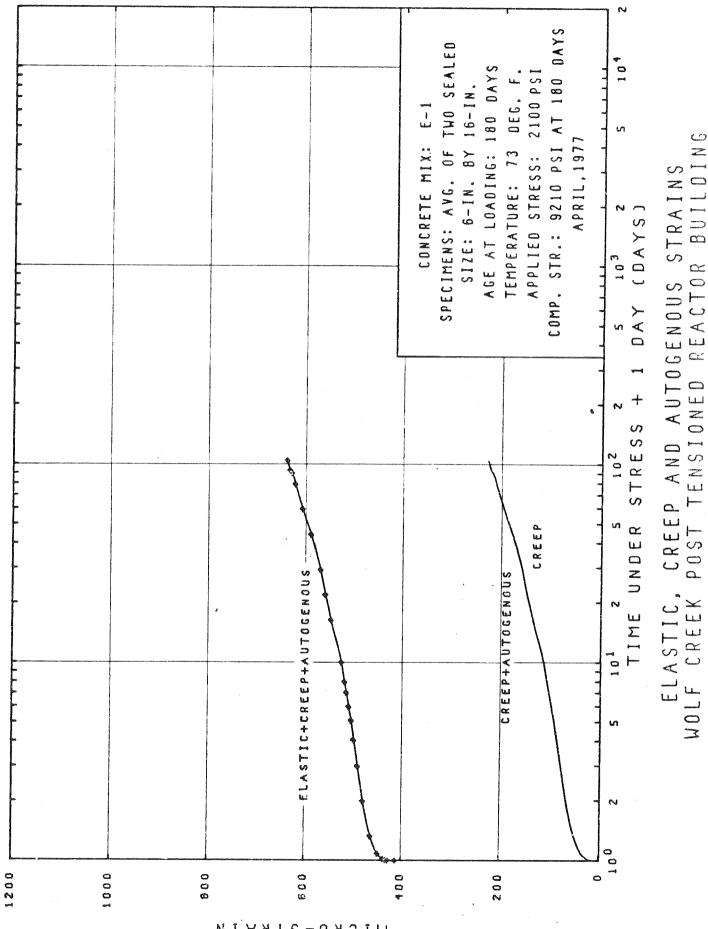


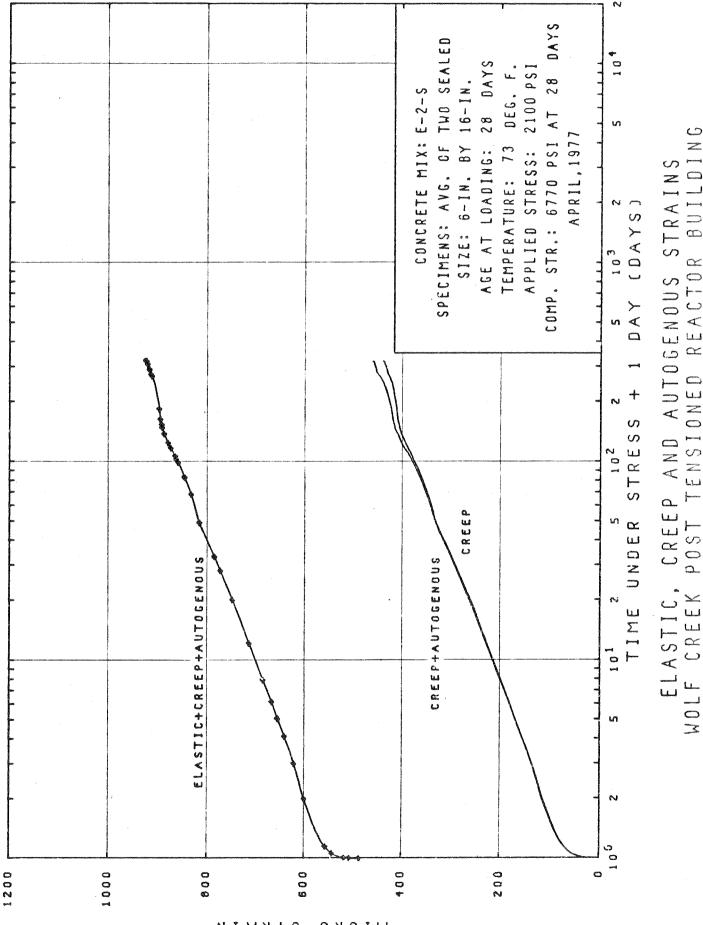
FIGURE 4

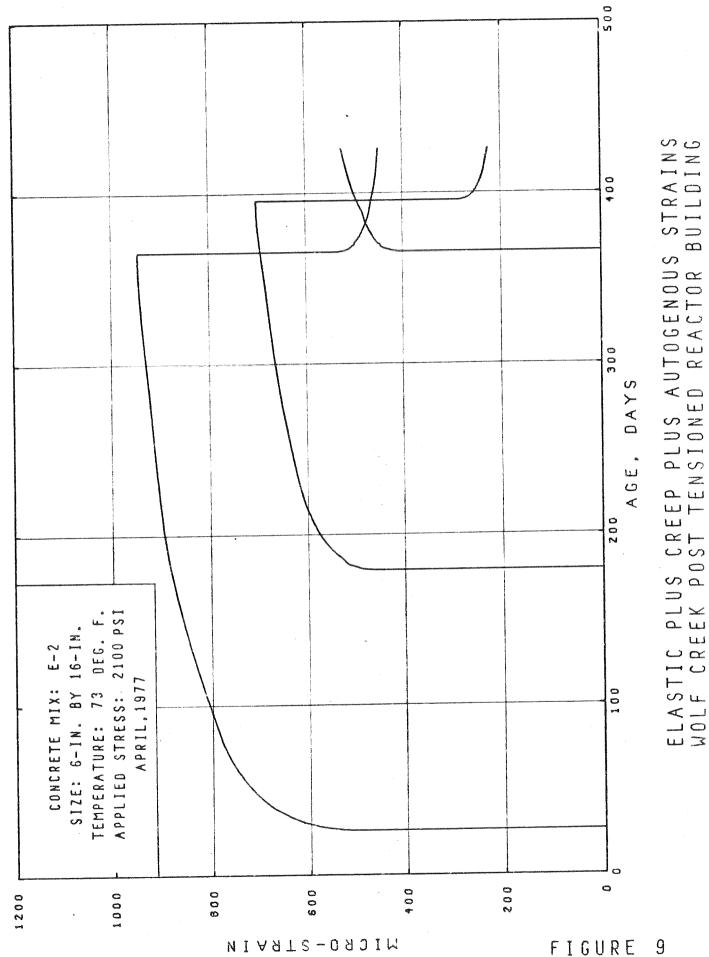
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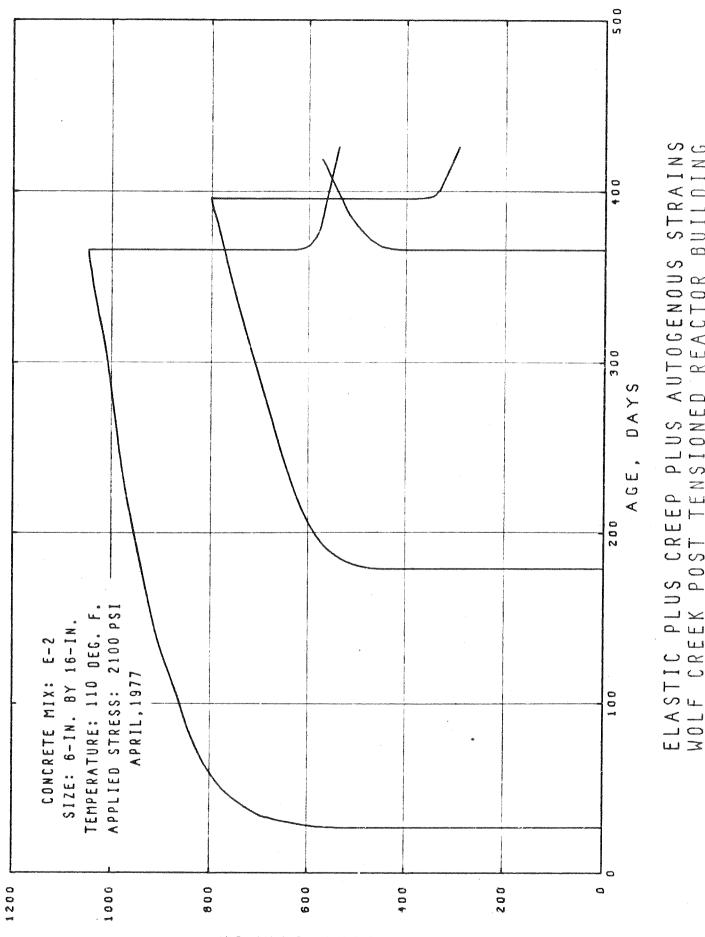








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