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1 **Can catchment-scale urban stormwater management measures benefit the stream**
2 **hydraulic environment?**

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17

18 **Abstract**

19 The potential for catchment-scale stormwater control measures (SCMs) to mitigate the impact
20 of stormwater runoff issues and excess stormwater volume is increasingly recognised. There
21 is, however, limited understanding about their potential in reducing in-channel disturbance and
22 improving hydraulic conditions for stream ecosystem benefits. This study investigates the
23 benefits that SCM application in a catchment have on in-stream hydraulics. To do this, a two-
24 dimensional hydraulic model was employed to simulate the stream hydraulic response to
25 scenarios of SCM application applied in an urban catchment to return towards pre-development
26 hydrologic pulses. The hydraulic response analysis considered three hydraulic metrics
27 associated with key components of stream ecosystem functions: benthic mobilization,
28 hydraulic diversity and retentive habitat availability. The results showed that when applied
29 intensively, the developed SCM scenarios could effectively restore the in-stream hydraulics to
30 close to natural levels. Compared to an unmanaged urban case (no SCMs), SCM scenarios
31 yielded channels with reduced bed mobility potential, close to natural hydraulic diversity and
32 improvement of retentive habitat availability. This indicates that mitigating the effect of
33 stormwater driven hydrological change could result in significant improvements in the physical
34 environment to better support ecosystem functioning. We therefore suggest that intensive
35 implementation of SCMs is an important action in an urbanizing catchment to maintain the
36 flow regime and hydraulic conditions that sustain the ‘natural’ stream habitat functioning. We
37 propose that stormwater management and protection of stream ecosystem processes should
38 incorporate hydraulic metrics to measure the effectiveness of management strategies.

39

40 **Keywords:** Urbanization, Stormwater management, Stream, Hydraulics, Stormwater runoff,
41 Urban hydrology

42

43 **1. Introduction**

44 Stream ecosystems are characterised by complex and dynamic ecosystem functions directly
45 governed by the hydraulic regime (Statzner & Higler, 1986; Kemp et al., 2000; Anim et al.,
46 2018a). In turn, patterns of hydraulic characteristics are determined by the interactions of flow
47 (i.e. magnitude, frequency, duration, rate of change and timing) and channel form (i.e. nested
48 features of topographic structure) (Jacobson & Galat, 2006). As a result, ecologists and river
49 scientists generally recognize the interactions between flow and form as a controlling template
50 for fluvial ecological processes (Townsend et al., 1997; Emery et al., 2003; Wallis et al., 2012;
51 Yarnell et al., 2015). Stream ecological integrity relies on the presence of natural dynamic
52 behaviour expressed through the hydraulic conditions (Statzner et al., 1988; Brooks et al.,
53 2005). Therefore, to sustain healthy natural stream ecosystem functioning, it is important to
54 maintain ecologically relevant hydraulic conditions that are similar to those in a naturally
55 functioning stream system.

56 When a catchment is urbanized, the sealing of native soils with impervious surfaces drastically
57 alters the water balance. Fluxes of evapotranspiration and infiltration are reduced and matched
58 by an increase in the surface runoff (i.e. urban stormwater) (Haase, 2009; Burns et al., 2013;
59 Fletcher et al., 2013). This excess water is typically managed by connecting impervious
60 surfaces to hydraulically efficient stormwater drainage systems which convey runoff directly
61 to streams draining the catchment (Roy et al., 2008; Walsh et al., 2012). When urban
62 stormwater runoff is directed to streams, many changes occur, including hydrological alteration
63 (Burns et al., 2012), water quality impairment (Brabec et al., 2002) and channel alterations
64 (Vietz et al., 2015). These changes to the flow regime and channel form unequivocally alter
65 the stream's hydraulic regimes (Jacobson & Galat, 2006; Anim et al., 2018a), resulting in
66 ecological degradation (Walsh et al., 2005; Paul & Meyer, 2008). Stormwater runoff is thus a
67 primary source of stress to stream ecosystems (Walsh, 2004; Ladson et al., 2006; Mallin et al.,
68 2009; Vietz et al., 2014; McIntyre et al., 2015).

69 To address this, increasing efforts have centred on stormwater management approaches that
70 aim to holistically mimic natural hydrological processes at the catchment-scale and treat
71 polluted runoff as well as deliver other benefits (e.g. improved amenity). Burns et al (2012)
72 coined such an approach the 'flow-regime stormwater management'. This approach
73 emphasizes the protection, restoration or mimicking of natural hydrological process at small

74 scales within the catchment, using stormwater control measures (SCMs), with the aim of
75 restoring natural flow regimes at larger scales downstream (Burns et al., 2012; Fletcher et al.,
76 2014). This catchment-focused approach agrees with the core principle of process-based
77 restoration that emphasize on addressing the root causes or source of degradation (Kondolf et
78 al., 2006; Beechie et al., 2010), such as urban stormwater runoff. Mitigating stormwater runoff
79 impacts requires that hydrologic objectives be specified, including 1) reducing the volume of
80 stormwater runoff, 2) restoring lost infiltration, and 3) returning the runoff response of
81 impervious surfaces towards the pre-development condition (Ladson et al., 2006; Walsh et al.,
82 2012; Burns et al., 2014). Such objectives can be achieved using specifically-designed SCMs
83 that are based on retention, detention, infiltration and harvesting of stormwater (e.g. DeBusk
84 et al., 2010a; Hunt et al., 2011; Li et al., 2017).

85 Several studies have tested and shown the potential hydrological performance of flow regime-
86 focused approaches to maintain or return the pre-development hydrological regime (e.g.
87 Damodaram et al., 2010; DeBusk et al., 2010b; Jenkins et al., 2012; Loperfido et al., 2014;
88 Burns et al., 2015a). For instance, stormwater bioretention systems (a common SCM) have
89 been tested and found to mimic pre-developed hydrologic performance (DeBusk et al., 2010b;
90 Davis et al., 2011), mitigating peak flows and total runoff volume (Winston et al., 2016; Liu &
91 Fassman-Beck, 2017). Jenkins et al. (2012) also showed that the hydrologic performance of
92 constructed stormwater wetlands led to significant runoff interception and mitigated total
93 runoff reaching the stream. The use of retention systems (e.g. rainwater tanks) has been found
94 to achieve stormwater retention performance comparable to pre-developed conditions by
95 reducing the frequency and volume of stormwater run-off from a site (Burns et al., 2015a).

96 Exactly how well the hydrologic outcomes of SCMs translate to the hydraulic needs of the
97 receiving stream ecosystem remains poorly understood. While understanding the hydrologic
98 outcome is important, it is critical to understand the anticipated translation into hydraulic
99 characteristics such as depth and velocity, which provide an explicit link to the habitat and
100 ecosystem functioning of the receiving streams (Clarke et al., 2003; Rosenfeld et al., 2011a).
101 Such consideration accounts for the interplay of streamflow dynamics with channel
102 morphology (Anim et al., 2018b), which drive habitat quality (Clarke et al., 2003; Escobar-
103 Arias & Pasternack, 2010). In particular, whilst bankfull discharge is often considered as
104 driving geomorphic change, it is increasingly recognised that the more subtle initial changes of
105 bed disturbance should be targeted for flow-regime strategies focused on the physical and
106 ecological changes of concern (Vietz & Hawley, 2018). Environmental flow management

107 approaches for sustaining stream ecosystem arguably have a better chance of maintaining
108 healthy ecological functioning when they are based on the mechanistic relationships between
109 flow and channel form (Clark et al., 2008; Yarnell et al., 2015). Therefore, an understanding
110 of how flow regime-focused approaches can protect or maintain the hydraulic conditions at or
111 near their natural levels is useful to inform strategies for urban stormwater management.

112 In this study we ask if flow regime-based stormwater management can restore in-stream
113 hydraulics to near their natural conditions. To test this, we used a two-dimensional (2D)
114 hydraulic model to simulate and examine the stream hydraulic responses to flow-regime
115 management strategies using different SCM scenarios applied in an urbanizing catchment.
116 Managing excess stormwater runoff as driver of stream ecosystem degradation is not
117 particularly a new thinking, but the novelty of this work is underpinned on the scope to
118 investigate the in-stream hydraulic outcomes of alternative approaches towards stormwater
119 management. More specifically, we aim to evaluate the effectiveness of the applied
120 management strategies to sustain the stream hydraulic conditions required for ecosystem
121 functioning in an urban catchment. To achieve this, the study first adopted a case-study natural
122 stream (with typical natural hydrology and channel form) in a natural catchment with no
123 development. Subsequently, various urban development scenarios with or without stormwater
124 management were explored.

125 **2. Methods**

126 *2.1 Experimental design*

127 To answer the study question, we formulated a modelling method made up of five parts (Fig.
128 1). Firstly, we adopted a case-study stream setting (with a typical natural hydrology and
129 channel form) in a natural catchment with no development. Secondly, a representative digital
130 terrain model (DTM) of the stream corridor topography was developed using existing field
131 channel reach parameters data and the synthetic river valley (SRV) methodology of Brown et
132 al. (2014). Thirdly, hydrological models were developed to produce different flow-regime
133 scenarios based on the (i) natural catchment with no development and (ii) developed catchment
134 with and without management (applied SCM alternatives). Fourthly, a 2D hydraulic model was
135 used to simulate the ecologically relevant hydraulic conditions delivered by each flow regime
136 scenario in the channel. Finally, temporally varying hydraulic patterns represented by metrics
137 of known link to relevant ecosystem functions were evaluated under each flow regime scenario.
138 We characterised the hydraulic patterns using three ecologically relevant hydraulic

139 characteristics: benthic disturbance; hydraulic diversity and retentive habitat availability, all of
140 which are important aquatic ecosystem drivers (Paterson & Whitfield, 2000; Brooks et al.,
141 2005; Vanzo et al., 2016). Details of each part are presented below.

142 *2.2 Case study setting: McMahons Creek catchment*

143 McMahons Creek catchment is located 90 km east of Melbourne (145.937°E, 37.821°S) with
144 a catchment area of 40 km². The catchment is forested throughout, mostly by mountain ash
145 (*Eucalyptus regnans*), with the lower slopes occupied by mixed species eucalypt forest and
146 riparian vegetation amounts to several percent of the total catchment area (Land Conservation
147 Council of Victoria, 1973). This remote catchment is not proposed for development but has
148 good flow records and is in close to natural condition. Physiography can be characterised by
149 steep terrain with partly confined channels (only pockets of floodplain within the valley sides).
150 Geologically, the catchment is largely covered by Devonian granites and sandstones, overlaid
151 by red and brown soils (Land Conservation Council of Victoria, 1973). The selected case-
152 study segment of the creek length has an intact and complex naturally meandering, pool-riffle
153 channel morphology comprised of well-sorted coarse-grained sediments with sand, gravels and
154 some boulders. Stream banks are commonly clay/silt with interbedded gravels between the
155 clay/silt layers. The channel morphology is comparable to typical naturally occurring shallow
156 streams in forested catchments in the Melbourne region. Rainfall pattern is fairly evenly
157 distributed over the year with an annual catchment rainfall averaging ~1000mm/year.

158 *2.3 Synthetic channel morphology*

159 An archetypal stream channel was designed for the McMahan Creek catchment in this study
160 using RiverBuilder package (version 0.1.0), an emerging technique of synthesizing channel
161 topography for science and engineering applications (Pasternack & Arroyo, 2018). Based on
162 the SRV mathematical framework of Brown et al. (2014), RiverBuilder is an open-source, free
163 R package capable of procedurally rendering a digital terrain model from user-selected
164 geometric functions that describe subreach topographic variability and associated parameter
165 values at reach and subreach scales. Methodological details are available in Brown et al. (2014),
166 and the information used to create the specific DTM used in this study is described here,
167 focusing on the two key steps at the reach and subreach scales.

168 2.3.1 Reach-average parameters

169 The SRV approach first creates a generic reach-average topography scaled by reach-average
 170 bankfull depth (H_{bf}) and width (W_{bf}), with median particle size (D_{50}), slope (S), sinuosity,
 171 floodplain width, and floodplain lateral slope as user-defined input parameters (Brown et al.,
 172 2014). Existing topographic data for the study stream segment in McMahons Creek provided
 173 reach-scale parameter values required to synthesize archetypal morphology (Table 1).

174 2.3.2 Channel variability parameterization

175 From the initial reach-average values above, RiverBuilder incorporates subreach-scale
 176 topographic variability using combinations of geometric functions at the user's expert
 177 discretion. The sub-reach variability for this study was created in the model according to Eq
 178 (1) and (2) such that the local bankfull width and bed elevation of the thalweg was estimated
 179 as

$$z_t(x_i) = (\overline{H_{bf}}f(x_i) + \overline{H_{bf}}) + S(\Delta x_i) + Z_d \quad (1)$$

$$W_{bf}(x_i) = (\overline{W_{bf}}f(x_i) + \overline{W_{bf}}) \quad (2)$$

180 where $z_t(x_i)$ and $W_{bf}(x_i)$ are local bed elevation and bankfull width at location x_i respectively.
 181 Z_d is the user-defined datum. The term $f(x_i)$ is the user-selected subreach variability function.
 182 Several possible functions are available in River Builder, such as linear, sinusoidal, and sine
 183 squared, depending on archetypal characteristics for a given class of stream. The general
 184 sinusoidal model was used to achieve the variability of W_{bf} and Z_t about the reach-averaged
 185 values by a control function $f(x_i)$ nested in Eqs. 2 and 3. The $f(x_i)$ was modelled as Eq (3):

$$y(x_i) = a_s \sin(b_s x_r + \theta_s) \quad (3)$$

186 where y_i is the dependent control function values, a_s , b_s , and θ_s as the amplitude, angular
 187 frequency and phase for the sinusoidal competent and x_r is the Cartesian stationing in radians
 188 (Brown et al., 2014). The resulting DTM of the channel morphology and the longitudinal
 189 profile is shown in Fig. 2.

190 Table 1. Channel reach-average and variability geomorphic attributes used in the design of
 191 the synthetic DTM. Reach channel parameters are field derived average values scaled from
 192 the case-study reach segment channel morphology.

Reach channel parameters			
Bankfull width (W_{bf}) (m)	6.5		
Bankfull depth (H_{bf}) (m)	0.8		
Median particle size (D_{50}) (m)	0.006		
Slope (S)	0.01		
Vertical datum (Z_d) (m)	1000		
Floodplain width (m)	10		
Floodplain lateral slope	0.005		
Channel length (m)	150		
Sinuosity	1.1		
Variability parameters			
	a_s	b_s	θ_s
Bankfull width	0.25	2	0
Planform	10	1	0
Bed elevation	0.25	2	0
Floodplain outline	5	1	3.14

193

194 2.4 Model development

195 Hydrologic modeling was performed using the Model for Urban Stormwater Improvement
 196 Conceptualisation (MUSIC) (eWater, 2015). MUSIC is commonly used for modeling
 197 stormwater flow and quality using continuous simulation (Schubert et al., 2017). In its default
 198 mode, MUSIC source nodes (which represent the catchments) use three rainfall-runoff stores:
 199 an impervious area store (describe by initial loss), a soil store (a linear reservoir described by
 200 infiltration and storage properties), and a groundwater store (a linear reservoir described by
 201 initial depth and daily rates of recharge, baseflow and seepage) (Hamel & Fletcher, 2014).
 202 MUSIC can model various SCM interventions such as rainwater tanks, infiltration and
 203 bioretention systems.

204 The model was first calibrated to observed flows under the current natural conditions. Model
205 parameters were then adjusted to simulate fully urbanized land use on the same catchment with
206 or without stormwater management (applied SCM alternatives) as described below. Further
207 details of model structure are reported in the Supplementary Material. This study used the July
208 2006 to July 2013 water years' data, which provides a good representation of dry, normal and
209 wet year conditions. Flow data, at a 6-minute timestep, were obtained from the McMahon's
210 Creek gauge (229106A) operated by Melbourne Water, while rainfall data were obtained from
211 the closest gauge (229102A) at Upper Yarra Dam. Calibration was undertaken for a range of
212 flow metrics covering the magnitude, timing and duration of flows, based on the approach
213 described by Hamel and Fletcher (2014). The model calibration is described in detail in Duncan
214 et al. (2016). After model calibration, the model scenarios were developed to represent different
215 cases of SCM implementation as described below

216 **Natural (pre-development) scenario:** This scenario represents the existing natural conditions
217 in the case study catchment. It forms the baseline for assessing the performance of the SCM
218 implementation strategies.

219 **Urban base scenario:** We then developed a model to simulate complete urbanisation of the
220 catchment according to typical urbanisation guidelines and practices in Melbourne (see Duncan
221 et al, 2016) without stormwater mitigation measures. In this scenario the impervious area
222 comprised 68% of the total catchment area, containing housing, roads and associated
223 impervious areas.

224 **SCM implementation scenarios:** Stormwater management scenarios were applied in MUSIC
225 to the urban base-case scenario with the aim of moving the flow regime back towards its pre-
226 development conditions. Management actions explored include diversion of ground-level
227 impervious runoff to bioretention systems, domestic and non-domestic water use from
228 rainwater tanks, diversion of tank overflow and controlled low-flow 'leaks' to bioretention and
229 harvesting of water from stormwater pipes upstream of watercourses for off-stream storage and
230 non-potable uses (e.g. landscape irrigation). We adopted three basic SCM scenarios, herein
231 labelled SCM30, SCM45 and SCM65, where the numbers (30, 45, 65) represent the target
232 percentage reduction in runoff volume). The overarching design objective was based on total
233 runoff volume reduction in comparison to the urbanised base case without SCM
234 implementation (Table 2). Scenario SCM30 uses only bioretention and rainwater tanks to
235 achieve a total 30% volume reduction. All ground level impervious runoff in this scenario was

236 directed to bioretention systems, while household roof runoff was directed to rainwater tanks
 237 which had a controlled slow-release to bioretention. Scenario SCM45 uses the same measures
 238 as SCM30, and in addition models the removal of additional 20% of the remaining runoff from
 239 stormwater pipes upstream of the watercourse, representing use for a range of non-potable
 240 purposes such as landscape irrigation, industry or agriculture. This scenario targeted an overall
 241 reduction in runoff volume of 45%. Scenario SCM65 uses the same measures as SCM45 but
 242 increases additional flow removal from 20% to 50% of runoff from stormwater pipes in every
 243 time step, thus achieving a total runoff volume reduction of 65%. Such a scenario might
 244 represent the case where stormwater was harvested, and treated, before being stored and used
 245 in the potable supply, as is already being trialled in some locations (e.g. McArdle et al., 2011)
 246 .

247 The MUSIC model outputs include flow time-series at 6-minute timestep, representing the flow
 248 regime of each modelled scenario (Fig. 3). Further details of the flow regimes of modelled
 249 hydrological scenarios are reported in the Supplementary Material.

250 Table 2. Volume reduction scenarios and pairs of basic SCMs used to retain volume reduction.
 251 See text for definition of scenarios acronyms.

Flow regime scenario	Volume reduction (%)	SCMs used to retain volume reduction
Natural	Natural	None
Fully urban	0	None (surface runoff directed to stream via stormwater pipes upstream of watercourse)
SCM30	30	Tanks and bioretention
SCM45	45	As FRM1 and 20% runoff removal from stormwater pipes
SCM65	65	As FRM2 and 50% runoff removal from stormwater pipes

252

253 2.5 Hydraulic modeling and scenarios

254 TUFLOW Classic is a numerical model that solves the full 2D (depth-averaged) momentum
 255 and continuity equations for free surface flow (Syms, 2001). It was used to simulate the

256 spatially explicit hydraulic patterns of the five flow regimes delivered from each of the
 257 hydrological scenarios described above. A square grid computational mesh was elevated with
 258 the RiverBuilder's DTM data points generated for the channel reach, with 150 longitudinal
 259 nodes spaced at 0.5 m ($\sim 1/16 W_{bf}$). The default TUFLOW Smagorinsky viscosity was used
 260 for turbulence closure with coefficient value of 0.5 and constant value of $0.005\text{m}^2/\text{s}$ suitable
 261 for shallow waters (e.g. Anim et al., 2018a). Manning's n was set to 0.05, representing typical
 262 unvegetated coarse-grained (gravel/boulders) surface roughness (Arcement and Schneider,
 263 1989). Typical of published exploratory numerical modeling studies, calibration of bed
 264 roughness or eddy viscosity was not possible as the study uses numerical models of theoretical
 265 channel archetypes in purely exploratory mode (e.g., Pasternack et al., 2008; Brown et al.,
 266 2016; Lane et al., 2018).

267 Model simulation input and exit boundary conditions included 10 flow stage and corresponding
 268 discharge (Q), ranging from 0.2-2.0x the bankfull flow (Q_{bkf}) stage (Table 3). Q_{bkf} stage is
 269 the water surface elevation (WSE) at which flow overtops the banks. Manning's equation was
 270 used to estimate the discharge values associated with the modelled flow stage based on
 271 representative cross-sections of the synthetic DTM (Table 3). Bankfull stage and wetted
 272 perimeter were calculated manually from the cross-sections and cross-sectional area
 273 determined using the parabolic approximation. These hydrological values used are scaled to
 274 the synthetic DTM to associate each modelled flow stage in the hydraulic model. We
 275 emphasize that these are estimates and should not be considered as utmost targets to inform
 276 management. 2D model outputs include hydraulic rasters of depth-averaged velocity in the
 277 direction of flow, water depth, bed shear stress (τ_b) and WSE. ArcGIS (Esri ArcGIS desktop
 278 10.2) was used to process and analyze these outputs to evaluate each investigated scenario.

279 Table 3. Channel archetype discharge values simulated for 0.2-2.0 times bankfull stage
 280 estimated using Manning's equation

Fraction of Q_{bkf} stage	0.2	0.4	0.6	0.8	1.0	1.2	1.4	1.6	1.8	2.0
Simulated Q (m^3/s)	0.18	0.73	1.64	2.62	3.83	5.86	8.35	10.90	13.17	17.15

281

282 2.6 Ecologically relevant hydraulic metrics

283 This study considered three eco-hydraulic relevant metrics associated with key components of
 284 stream ecosystem functions: variation of benthic disturbance that affect bed mobilization and
 285 drift of benthic biota that lives in them (e.g. Gibbins et al., 2010); variation of hydraulic
 286 diversity (e.g. Gostner et al., 2013); and physical habitat availability (e.g. Vietz et al., 2013).
 287 Quantitative hydraulic performance metrics related to these ecosystem functions used includes:
 288 (i) established near-bed Shield stress thresholds as indicators of bed mobility, (ii) a measure of
 289 spatial heterogeneity of flow depth and velocity that reflects overall reach hydraulic diversity
 290 and (iii) a measure of retentive habitat area that quantifies availability of slow and shallow
 291 depth water. We examined these hydraulic functions using an ArcGIS decision tree that
 292 enabled rapid evaluation of the hydraulic model raster outputs over specific defined threshold
 293 bounds.

294 2.6.1 Benthic disturbance

295 Benthic space is naturally disturbed by bed material movement in unaltered hydrological
 296 regime reaches on a periodic basis, but this process has been shown to increase in magnitude,
 297 frequency and duration with urbanization. This increases streambed instability and degradation
 298 (e.g. Hawley and Vietz, 2016; Anim et al., 2018a) and impacts biota (Hawley et al., 2016). The
 299 non-dimensionalized bed shear stress known as Shields stress (τ^*) was used to quantify the bed
 300 mobility potential of the channel in each grid cell of the model. The shields stress was
 301 calculated as:

$$\tau^* = \frac{\tau_b}{D_{50}(\gamma_s - \gamma_w)} \quad (4)$$

302 where τ_b is bed shear stress and γ_s and γ_w are the unit weight of bed particle and water
 303 respectively. In this study, τ^* values were classified based on established bed particle mobility
 304 threshold, where $\tau^* < 0.03$ indicates stable bed or no mobility, $0.03 < \tau^* < 0.06$ indicates partial
 305 mobility (i.e. incipient motion of finer particles at the bed surface) and $\tau^* > 0.06$ indicate full
 306 bed mobility (i.e. persistent movement of a sheet of bed particles) (Wilcock and McArdeell,
 307 1993; Buffington and Montgomery, 1997; Sawyer et al., 2010). The mobility performance was
 308 then quantified as the cumulative proportion of the channel bed experiencing the different
 309 levels of mobility as defined by the threshold. The results were then binned for comparison
 310 purposes such that low, medium, and severe disturbance are associated with 0-20%, 20-50%
 311 and above 50% proportion of the channel bed experiencing at least partial bed mobility

312 respectively. For instance, above 50% of the channel bed area must be experiencing at least
 313 partial or full bed mobility to be considered severe disturbance.

314 2.6.2 Hydraulic diversity

315 Varying patterns of flow velocity and depth have been recognized as part of the stream
 316 heterogeneity key to ecosystem integrity (Rosenfeld et al., 2011b). Hydraulic variability
 317 supports differentiation of species' life history strategies (Verberk et al., 2008; Braun and
 318 Reynolds, 2014). We used the hydro-morphological index of diversity (HMID) developed by
 319 Gostner et al. (2013) to quantify the overall hydraulic diversity in the channel for a given
 320 discharge. The HMID is based on the reach-scale coefficient of variation (CV) of flow velocity
 321 (u) and water depth (d) estimated as:

$$HMID_{channel} = (1 + CV_u)^2 + (1 + CV_d)^2 \quad (5)$$

322 where $CV = \sigma/\mu$, σ and μ are the standard deviation and mean value respectively. Results
 323 were binned to reflect Gostner et al. (2013) proposal such that $HMID < 5$ assumes low diversity;
 324 $5 < HMID < 9$ assumes medium or transitional diversity; $HMID > 9$ assumes high diversity.

325 2.6.3 Retentive habitat availability

326 Shallow slow-water habitat (SSWH) area was used to evaluate the relative habitat availability
 327 for explored scenarios. SSWH are vulnerable to an altered hydrological regime. Decreases in
 328 SSWH area impact fish abundance, macroinvertebrates that rely uses such habitat for refugia,
 329 and organic matter retention (Schiemer et al., 2001; Vietz et al., 2013). SSWH (total area per
 330 channel length) was estimated from the model flow depth and velocity raster using an ArcGIS
 331 python script that processes water depth and velocity outputs to locate cells with joint velocity
 332 and depth values of 0-0.2 m/s and 0-0.3 m respectively. This depth and velocity criteria is
 333 particularly preferred by fish (Milhous and Nestler, 2016) and benthic macroinvertebrates
 334 (Shearer et al., 2015) in streams.

335 2.7 Hydraulic regime performance analysis

336 An approach that blends hydrological time series with functional hydraulic performance was
 337 employed to evaluate the hydraulic response of each explored flow regime scenario. First,
 338 functional relationships were developed for the full range of flows modelled (Table 3) for each
 339 hydraulic metric investigated. Then the functional relationships were integrated with flow time
 340 series of each hydrologic scenario to yield hydraulic metric time series. The resulting annual
 341 time series represent the temporal pattern of the hydraulic response under each hydrologic

342 scenario. The relative influence of each flow scenario to maintain or restore stream hydraulics
343 regime was evaluated by quantitatively characterizing and comparing the temporal variation in
344 each explored hydraulic metrics to the pre-development conditions. This approach employed
345 simple descriptive statistics, where the statistical analysis of the time-series of each metric
346 aimed to evaluate the relative percent change of the various aspects of the hydraulic behaviour.
347 This includes frequency, magnitude and duration, which are key elements of the hydraulic
348 template (Poff and Ward, 1990). The analysis also considered the increase or decrease of the
349 metrics as a function of discharge relative to the explored scenarios where the degree of change
350 was examined corresponding to the defined thresholds.

351 **3. Results**

352 *3.1 Variability of hydraulic metrics with discharge*

353 The model results showed a decrease in the portion of the channel benthic area experiencing
354 no bed mobility ($\tau^* < 0.03$) beginning as the flow reaches approximately $0.4 Q_{bkf}$ (Fig. 4a).
355 This represents flows of 65% and 85% exceedance for both developed and pre-developed mean
356 daily flow regime respectively. A slight decrease was then observed as flows near $0.9 Q_{bkf}$ and
357 tends to stop as flows spills over the banks reaching a constant 15% with over 50% of the
358 channel bed under partial or full mobility. A predicted ~75% of the wetted channel bed area
359 experienced either partial or full bed mobility at bankfull flow.

360 The HMID values invariably decreased as flow increased and eventually stabilised once flow
361 spilled over the banks (Fig. 4b). HMID was substantially higher at baseflows ($< 0.2 Q_{bkf}$) than
362 higher flows ($> 0.5 Q_{bkf}$), with baseflow values about 4x as high. Above $0.2 Q_{bkf}$, the HMID
363 values reduce and attains medium values ($5 < \text{HMID} < 9$). It then transitions from medium to
364 low values ($\text{HMID} < 5$) as flows reaches $0.5 Q_{bkf}$ and tends to stop around an approximately
365 constant value of $\text{HMID} = 2$ for very high flows. Here, larger flow depth and velocity CV for
366 flows below $0.2 Q_{bkf}$ was observed. Generally, mean flow velocities and water depth ranged
367 from 0 to 1.1 m/s and 0 to 0.83 m respectively. At Q_{bkf} , maximum velocity and depth were 1.5
368 m/s and 1.28 m respectively. The water depth was observed to change rapidly at low flow
369 variations ($< 0.3 Q_{bkf}$), whereas the flow velocity was sensitive to variations in high flows (> 0.5
370 Q_{bkf}).

371 The SSWH area initially increased gradually with complete wetting conditions of the active
 372 channel bed topography as flow increased (Fig. 4c). This was associated with low flows up to
 373 $0.2 Q_{bkf}$, beyond which the SSWH area diminished rapidly and was near zero at Q_{bkf} . Once
 374 flows overtopped the banks, there was a substantial increase in SSWH area as floodplains were
 375 inundated.

376 3.2 Hydrologic scenarios comparisons

377 3.2.1 Benthic disturbances

378 The urban baseline scenario produced the most unstable bed within the channel, dominated by
 379 increased periods of the channel bed experiencing either partial or full mobility (Fig. 5). The
 380 predicted frequency and magnitude of portion of the benthic space that was exhibiting severe
 381 benthic disturbance (period that over 50% of the channel bed area shows partial or full bed
 382 mobility) were substantially greater. The influence of flow alteration in the urban hydrological
 383 regimes was revealed in the frequency and duration of the severe disturbance (Fig. 6a and 6b).
 384 For example, comparing the natural flows and urban flows the frequency (number of days) that
 385 channel bed areas experience severe disturbance under urban scenario was about 50x that of
 386 the natural (pre-developed) state. This was estimated to be 217 days for the study period,
 387 averaging 37 days/year. This represents ~8% of the total study period compared to 0.1% for
 388 the natural. It reflected the increased frequent-high magnitude storm flows with the altered
 389 hydrology. In other words, almost all the estimated days of channel experiencing severe
 390 disturbance under urban scenario were associated with flows occurring ~40% of the time. In
 391 contrast, the natural flow regime resulted in predominantly stable bed most of the year,
 392 averaging 2 days/year of severe disturbance, with approximately 85% and 14% of low and
 393 moderate disturbance respectively.

394 The different SCM approach interventions (SCM30, SCM45, SCM65) showed reduced
 395 potential benthic disturbance compared to the fully urban. The observed periods of severe
 396 disturbance compared to the urban scenario were substantially improved particularly for
 397 SCM65 which showed a benthic disturbance regime close to the natural scenario. The
 398 estimated period under severe disturbance plummeted from 8% under urban scenario to 5%,
 399 2.7% and 0.4% under SCM30, SCM45 and SCM65 respectively. More importantly, the
 400 temporal analysis revealed that, the continuous duration of the channel bed exhibiting severe
 401 disturbance of greater than 2 days was substantially reduced with SCMs applied (Fig. 6b).
 402 Similar to the natural scenario, the duration of period of severe disturbance in the channel bed

403 under SCM65 was short-lived, mostly within 0-2 days. Here, the period of severe disturbance
404 was only larger for long duration-high magnitude flows occurring ~3% of the time of the flow
405 regime.

406 3.2.2 Reach hydraulic diversity

407 The temporal hydraulic diversity pattern was highlighted by the HMID exceedance curves for
408 all investigated scenarios (Fig. 7). For all scenarios, HMID values were within moderate to
409 high bins about 75% of the time. The natural scenario produced higher HMID values for most
410 of the year showing higher temporal persisting diverse in-channel hydraulics, with values
411 within medium to high performance equalled or exceeded ~95% of the time. Under urban
412 scenario, high HMID values (>9) occurred only 20% of the time with marginally higher (>11)
413 values compared to all other hydrologic scenarios exceeded 10% of the time. This is related to
414 the extended lower summer and winter baseflows under urban scenario. For flow regimes under
415 SCM intervention scenarios (SCM30, SCM45, SCM65), HMID values showed some
416 improvement in the temporal hydraulic diversity compared to urban scenario, particularly for
417 SCM65 (which yielded an HMID regime close to that of natural scenario).

418 The natural scenario shows consistently high HMID values across the year, particularly during
419 winter period (June-August) when frequent storms flows are expected (Fig. 8). SCM65
420 exhibited a similar HMID pattern to natural scenario. Sensitivity of the hydraulic diversity to
421 frequent flow alteration was illustrated for urban scenario, when HMID values fluctuated
422 rapidly between low, medium and high performance.

423 3.2.3 Retentive habitat availability

424 SSWH area exceedance curves revealed a substantial reduction of the temporal persistence of
425 SSWH availability in the channel under the urban flow regime (Fig. 9). This was up to about
426 3x less relative to the natural scenario for the total study duration, particularly for flows
427 between 40-60% exceedance. Considering the median of these flow regimes (Table 4), the
428 urban scenario reduces SSWH availability on average by approximately 30-45% annually for
429 the study period. As low flows produce higher SSWH availability in general, it is unsurprising
430 that urban scenario exhibited slightly higher SSWH areas occurring about 20% of the time,
431 related to the extended lower baseflows.

432 The influence of flow alteration in the urban flow regimes was also revealed in the frequency
433 distribution of SSWH availability per unit 150 m over the study duration (Fig. 10). For
434 example, comparing natural and urban scenarios showed a reduction in the frequency (number

435 of days) of larger areas of SSWH ($>200 \text{ m}^2/150 \text{ m}$). Under natural scenario, diversity in the
 436 SSWH areas is greatest with larger areas of SSWH frequently present. Smaller areas of SSWH
 437 ($< 100 \text{ m}^2/150 \text{ m}$) are most common under altered hydrological regimes particularly for the
 438 urban scenario which skews the distribution further. Overall, reductions of SSWH availability
 439 was minimized by the alternative SCM scenarios (SCM30, SCM45 and SCM65). The applied
 440 SCMs appropriately improved the totally skewed to very little SSWH areas commonly
 441 occurring under urban scenario towards the natural scenario. This improvement was most
 442 evident for SCM65, which retained a total SSWH areas close to the natural scenario, with only
 443 marginal reduction in the magnitude and duration of SSWH areas. Conversely SCM30 had
 444 little effect on the frequency and magnitude of larger SSWH areas.

445

446

447 Table 4. SSWH area of each modelled scenario at median flows

Modelled scenario	Flow (m^3/s)	SSWH area (m^2)
Natural	0.188	240.6
SCM65	0.177	255.2
SCM45	0.247	161.2
SCM35	0.294	143.3
Urban	0.381	130.0

448

449 **4. Discussion**450 *4.1 Hydraulic effects of an urban-induced altered hydrologic regime*

451 As demonstrated in this study, the altered flow regime that results from urbanization drives
 452 fundamental deleterious changes to the natural hydraulic regime of the stream ecosystem. This
 453 coincides with widely recognized arguments made by researchers that urban stormwater runoff
 454 is a major stressor to urban stream ecosystems (Brabec et al., 2002; Walsh, 2004; Ladson et
 455 al., 2006; Burns et al., 2012; Vietz et al., 2014). In turn, this is a primary contributor to
 456 decreased ecological health often observed in streams draining urban catchments (Wenger et
 457 al., 2009; Groffman et al., 2014).

458 The results suggest that the urban flow regime could lead to the channel experiencing
459 substantially higher bed mobility, making the channel bed highly unstable, the first stage to
460 channel incision (Hawley and Vietz, 2016). Full transport defined by Sawyer et al. (2010) as
461 persistent entrainment of a sheet of bed particles will occur more frequently and for longer
462 durations following urbanization, given that urban hydrology is characterized by increased
463 frequency, magnitude and volume of storm flows (Anim et al., 2018b). This means acceleration
464 of channel evolution processes, including deleterious positive feedback such as containment of
465 greater volumes of streamflow once channel capacity increases (Vietz and Hawley, 2018).

466 Increased frequency, duration, and spatial extent of bed mobility in this degradation mechanism
467 translates to ecological impacts via regular disturbance of physical habitat (Francoeur and
468 Biggs, 2006) and eventually habitat loss, limiting benthic refuge space (Negishi et al., 2002).
469 Benthic disturbance dynamics is a key factor in the distribution, abundance and diversity of
470 benthic biota (Townsend et al., 1997). This type of disturbance does not yield a consistent
471 regime that species can adapt or acclimatise to.

472 Sensitivity of spatial and temporal hydraulic diversity to the flow alteration after urbanization
473 has relevant implications for biodiversity and ecosystem functioning. While the channel
474 maintained temporal persistence of high range and coefficient of variation of depth and velocity
475 for the most part, altered hydrology in the urban case increases the magnitude and frequency
476 of higher discharge events. This renders the channel liable to frequent fluctuations of hydraulic
477 diversity, with limited temporal persistence of the larger range and covariance of depth and
478 velocity. Gostner et al. (2013) argued that for channels experiencing such rapid fluctuations,
479 the chances of maintaining a healthy biotic stream community are limited. While a higher
480 hydraulic diversity alone does not necessarily yield a healthy stream or suitable ecological
481 performance, it is expected to impact the longitudinal distribution and assemblages of biota
482 (Elosegi et al., 2010; Lane et al., 2018).

483 In addition, retentive habitat availability under the urban hydrological regime is low, limiting
484 opportunities for biotic refuge. Persistent limited availability of SSWH can reduce breeding
485 and rearing habitat and refuge which could be a major factor for local extinction and reduced
486 assemblages and diversity of biota (Poznańska et al., 2009; Wenger et al., 2009; Koperski,
487 2010).

488 These factors suggest that appropriate urban flow regime stormwater management is a likely
489 requirement to protect the hydraulic conditions of streams. Excess stormwater runoff volume

490 needs to be prevented from becoming streamflow to have a chance of sustaining the ecosystem
491 functioning.

492 *4.2 Can catchment-scale application of SCMs restore a more natural hydraulic condition?*

493 Our results demonstrate that a high level of SCM implementation is necessary to maintain in-
494 stream hydraulic conditions close to pre-development levels in urban catchments. Similar to
495 what is proposed to restore and/or protect geomorphic form (Vietz et al., 2015), water quality
496 (Fletcher et al., 2014) and ecology (Walsh et al., 2015). The hydraulic performance of the SCM
497 scenarios (SCM30, SCM45, SCM65), compared with the natural scenario suggest that
498 protecting or restoring ecologically relevant aspects hydraulic regime through catchment-scale
499 application of SCMs is feasible, but requires relatively high levels of SCM intervention. The
500 hydraulic behaviour of adopted hydraulic performance metrics showed that the three designed
501 SCM scenarios could potentially reduce the impact of stormwater runoff on the stream
502 ecosystem. The volume reduction achieved is an important surrogate predictor of the changes
503 to each of hydraulic metrics, with the most effective scenario being SCM65. The SCM30
504 scenario provided only marginal improvement of the hydraulic conditions.

505 The observation that intensive application of SCMs is necessary to fully protect the hydraulic
506 environment has important implications for stormwater management. In reality, achieving such
507 volume reductions will need to involve significant harvesting; relying on infiltration or
508 evapotranspiration alone will not be sufficient (Walsh et al. 2016). In essence, the design
509 stormwater control measures should have the capacity to retain rainfall up to the amount that
510 would have caused widespread surface runoff under natural catchment conditions (Burns et al.,
511 2015b). As an example, in south-eastern Australia, this amount has been calculated as being
512 around 25 mm (Hill et al., 1996).

513 Several authors have demonstrated that achieving such an outcome requires that SCMs be
514 applied at or near source throughout the catchment (e.g. Meyer and Wallace, 2001; Burns et
515 al., 2015a; Walsh et al., 2016), as this provides greater opportunity to mimic natural flow paths
516 and restore a natural water balance. By this reasoning, we posit that, it is possible for
517 urbanization to be managed with suitable infrastructure to avoid significant impact on the in-
518 stream hydraulic conditions. Management interventions to achieve such large volume
519 reduction include diversion of ground level impervious runoff to bioretention systems,
520 domestic and non-domestic water use from rainwater tanks, diversion of tank overflow and
521 controlled low-flow 'leaks' to bioretention and harvesting of water from stormwater pipes

522 upstream of the stream for offstream storage and use (e.g. DeBusk et al., 2010a; Burns et al.,
523 2015a).

524 Our modelling suggests that lower levels of implementation of SCMs are unlikely to provide
525 the natural hydraulic conditions, as demonstrated by the SCM30 scenario. This suggests that
526 partial hydrological regime restoration in an established urbanized catchment may not be
527 enough to protect the hydraulic environment.

528 *4.3 Challenges of appropriate scale for flow-regime stormwater management*

529 Achieving high levels of volume reduction could be challenging, especially in an established
530 urban catchment, due to space constraints and limited demand for alternative water supplies
531 (Hamel et al., 2013; Walsh et al., 2016). In a retrofit situation, there will be a large cost required
532 for retention and storage (see for example Burns et al., 2015a; Li et al., 2017), but it is worth
533 noting that such strategies also bring other benefits such as improving urban amenity through
534 increased social values and enhancing the urban microclimate (Roehr and Fassman-Beck,
535 2015; Kuller et al., 2017). Considering these challenges, it is clear that implementation will be
536 most feasible when it is planned at the development phase, where there is the potential to
537 incorporate the required SCMs and water harvesting as part of the construction phase, both
538 reducing net cost and maximising the other secondary benefits provided (Walsh et al., 2016).

539 *4.4 Opportunities for management to protect stream ecosystem*

540 The results of this study suggest that the definition of urban stormwater management for stream
541 protection should require meeting objectives that maintain the natural hydraulic regime of
542 receiving streams. In this context, hydraulic performance metrics provide useful and specific
543 design objectives for SCM implementation. Recent studies have contended that streamflow
544 considerations should go beyond hydrologic assessment and include hydrogeomorphic
545 evaluations that provide a better understanding of the effects of intended management actions
546 (Wohl et al., 2015; Yarnell et al., 2015; Stone et al., 2017). Hydraulic conditions provide an
547 explicit mechanistic linkage between exogenous variables and ecological responses and are
548 associated with key components of stream ecosystem integrity: hydrogeomorphic processes
549 and aquatic habitat (Kemp et al., 2000; Escobar-Arias and Pasternack, 2010; Vanzo et al.,
550 2016). This is in line with the guidance of Walsh et al. (2016), who argue that a target for the
551 ecological state of the stream ecosystem to be protected should be identified and used to set
552 performance objectives for catchment-wide stormwater management.

553 **5. Conclusions and future works**

554 Stream ecosystem processes are substantially governed by their hydraulic regime, which in
555 turn is driven substantially by catchment hydrology. This study examined how catchment-wide
556 application of stormwater control measures implemented focused on restoring more natural
557 flow regimes in an urbanizing catchment could maintain or restore in-stream hydraulics
558 towards their pre-development conditions. By investigating quantitative eco-hydraulic metrics,
559 we were able to evaluate the hydraulic response to changes in the hydrological regimes.
560 Comparing the performance of these metrics suggested that SCM implementation is a
561 prerequisite to sustaining the hydraulics at pre-development levels to protect the ecological
562 structure and function.

563 The results highlighted that stormwater management that maximises the retention, harvesting
564 and infiltration of surface runoff would have noticeable impact if applied intensively
565 throughout the catchment, such that the runoff volume approaches that which would have
566 occurred prior to urbanisation. Given that the managed flow regimes should result in suitable
567 hydraulic conditions for ecosystem functioning, we propose that stormwater management and
568 protection of stream ecosystem processes should target strategies for and incorporate
569 anticipated effects on stream hydraulics. Our study provides a novel framework for more
570 quantitative assessment of the effectiveness of stormwater management strategies, using
571 hydraulic metrics associated with key elements of stream ecosystem functions.

572 Our study has emphasized the need for a large proportion of surface runoff to be prevented
573 from becoming streamflow. We acknowledge this will be challenging in terms of the space
574 required, cost, and finding demand for the harvested stormwater, particularly for an established
575 urban catchment. Such challenges should be weighed up, however, against the range of other
576 benefits to urban amenity that result from returning a more natural water balance in urban
577 landscapes.

578 Further work is needed to identify specific hydraulic metrics that could guide design in
579 particular streams, based on channel form, substrate composition, or ecological values to be
580 protected. In addition, in cases of established urban catchments, where restoring altered flow
581 regimes is difficult, further research would be useful to understand how the target stream's
582 channel form influences the effect of altered hydrology on key stream ecosystem functions.
583 Understanding the template of hydraulic conditions that results from the interplay between
584 channel form and flow could help to design complementary channel modification. Independent

585 adjustment of flow and channel form might give managers additional flexibility for
586 ecologically successful restoration and protection of streams in urban catchments.

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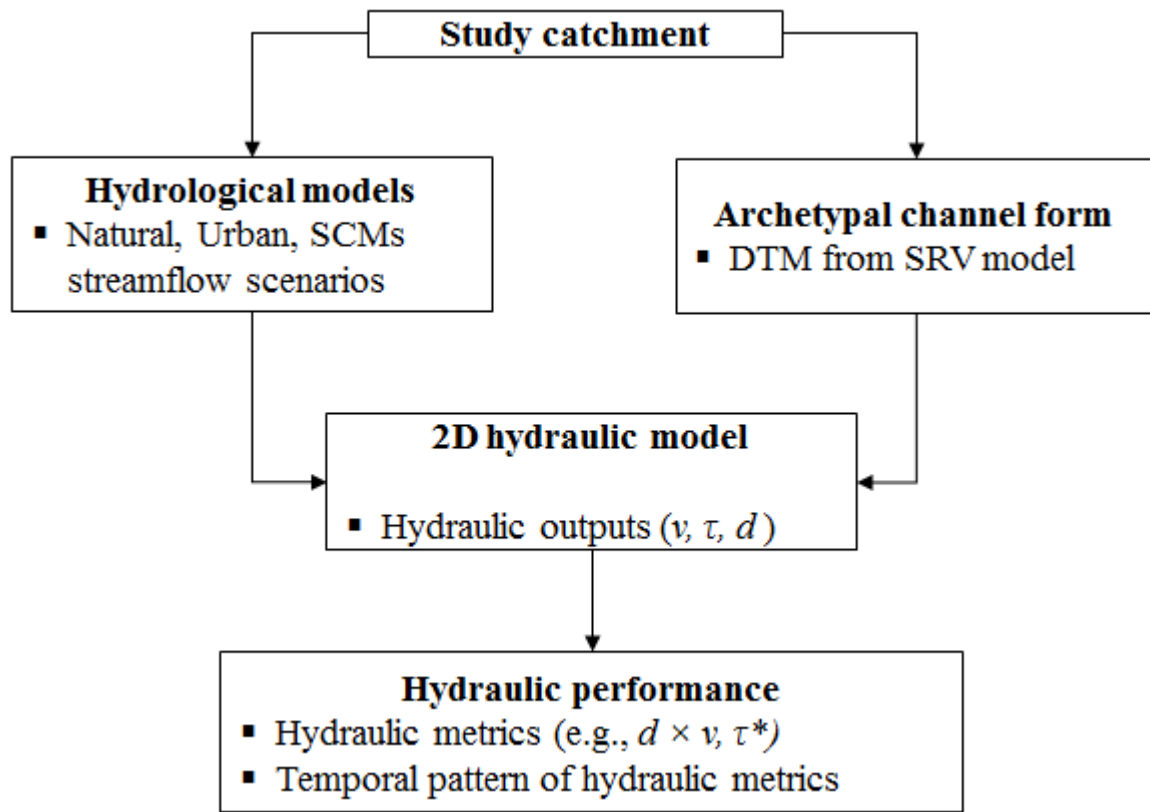
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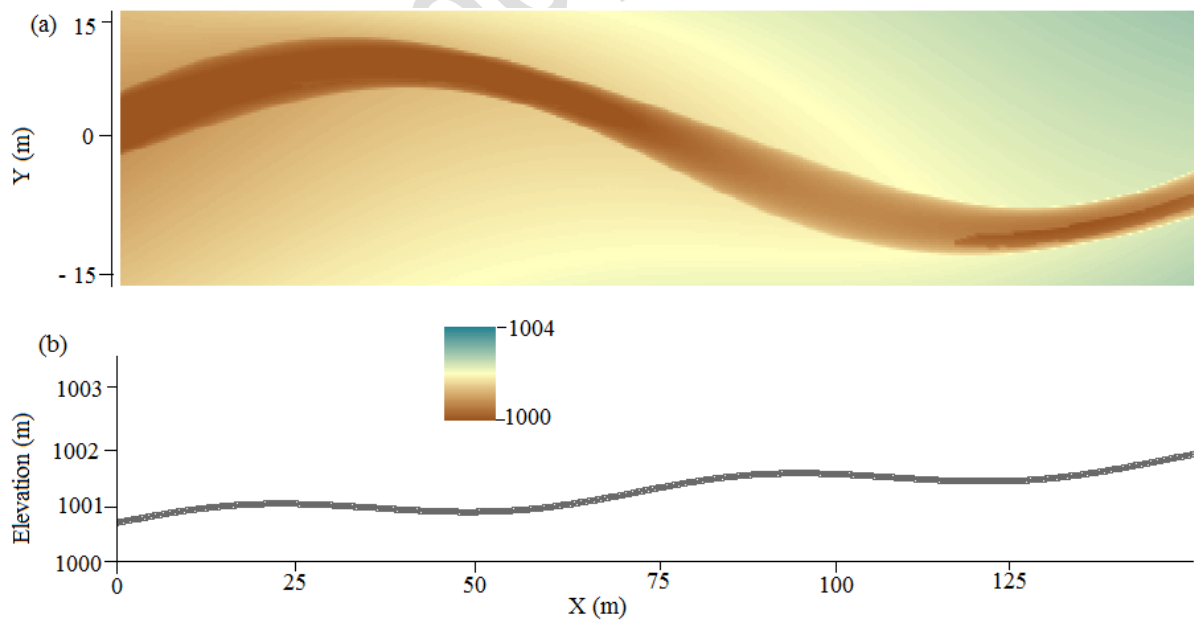
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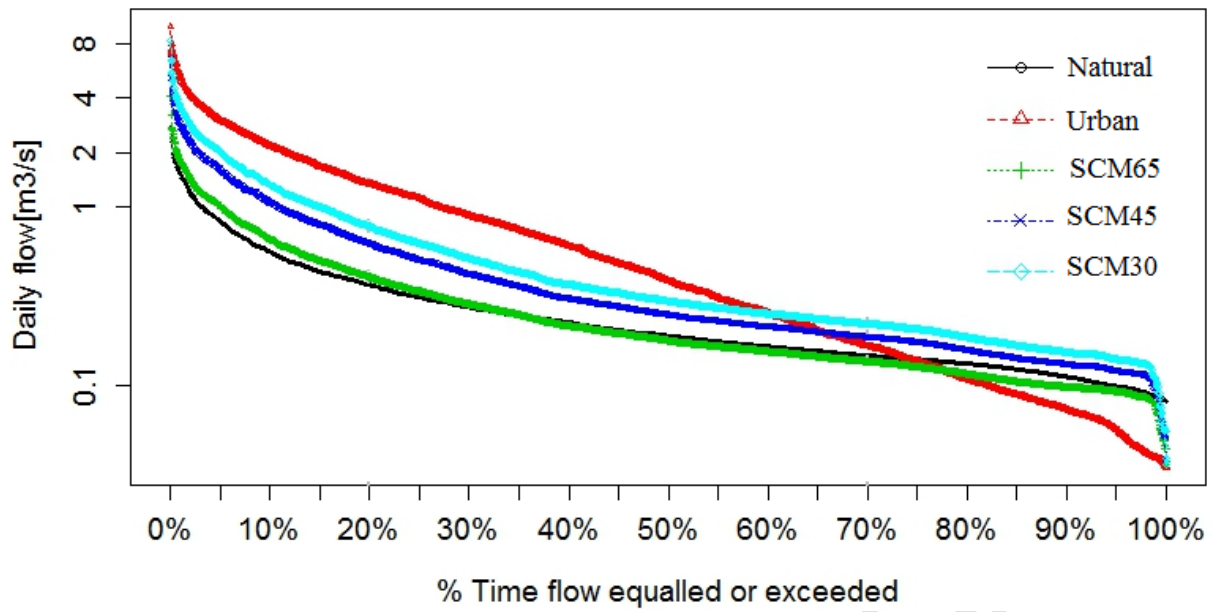
873 Fig. 1. Steps followed to quantify hydraulic performance of each explored flow-channel form
874 scenario



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876 Fig. 2. (a) The synthetic DTM and (b) the longitudinal profile of the thalweg.

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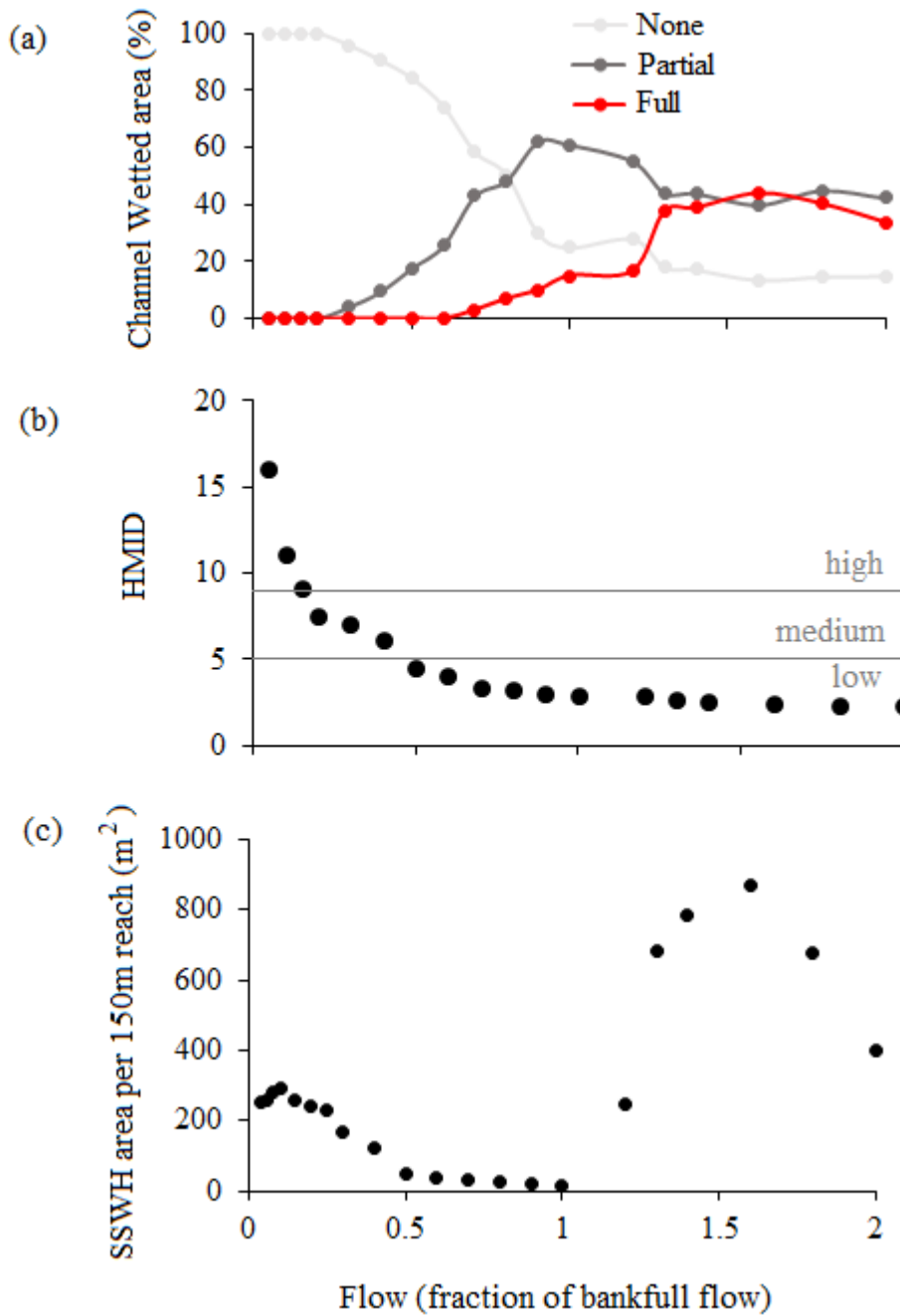


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879 Fig. 3. Flow duration curves that summarised the modelled time-series (daily) for each
880 scenario.

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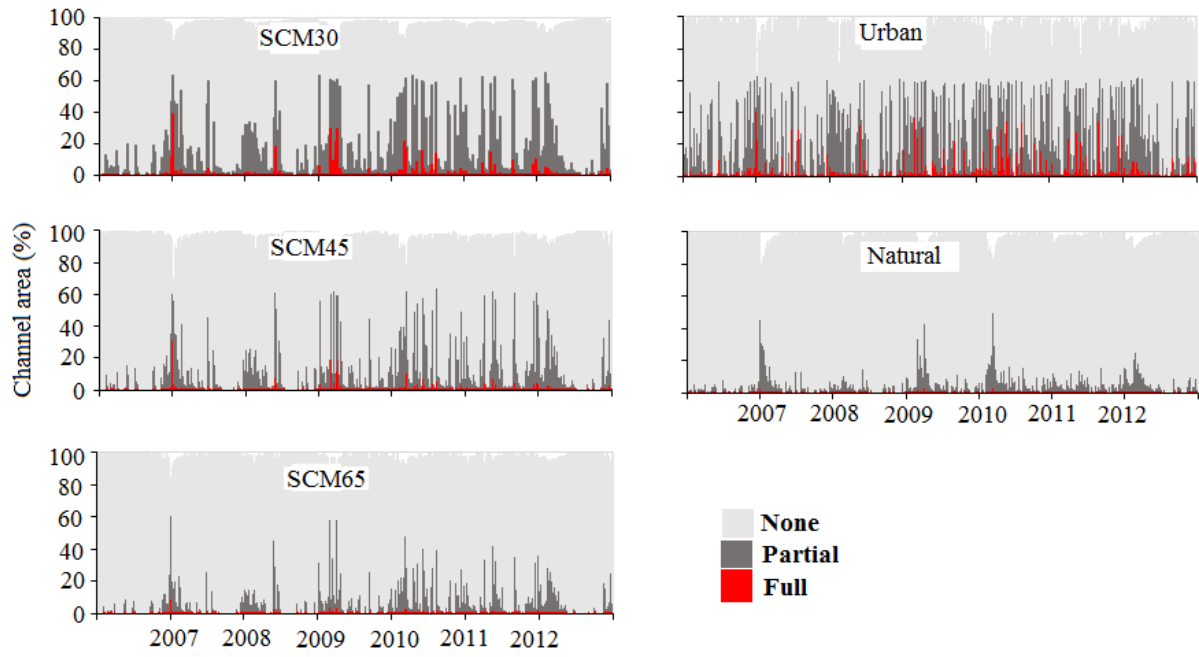


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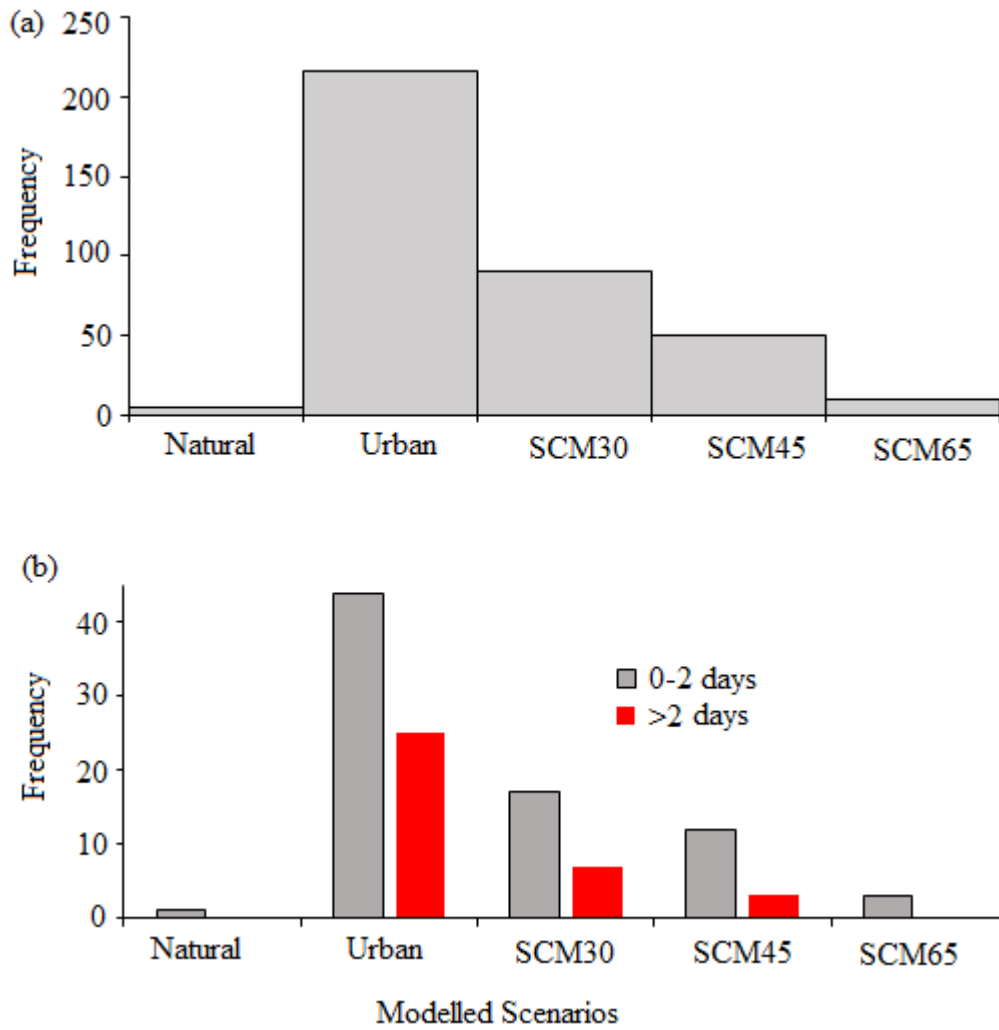
883 Fig. 4. Relationship between discharge (as a fraction of bankfull flow) and hydraulic metrics.

884 (a) Proportion of the wetted channel bed area under different classification of sediment

885 mobility, (b) HMID values, and (c) SSWH area values.

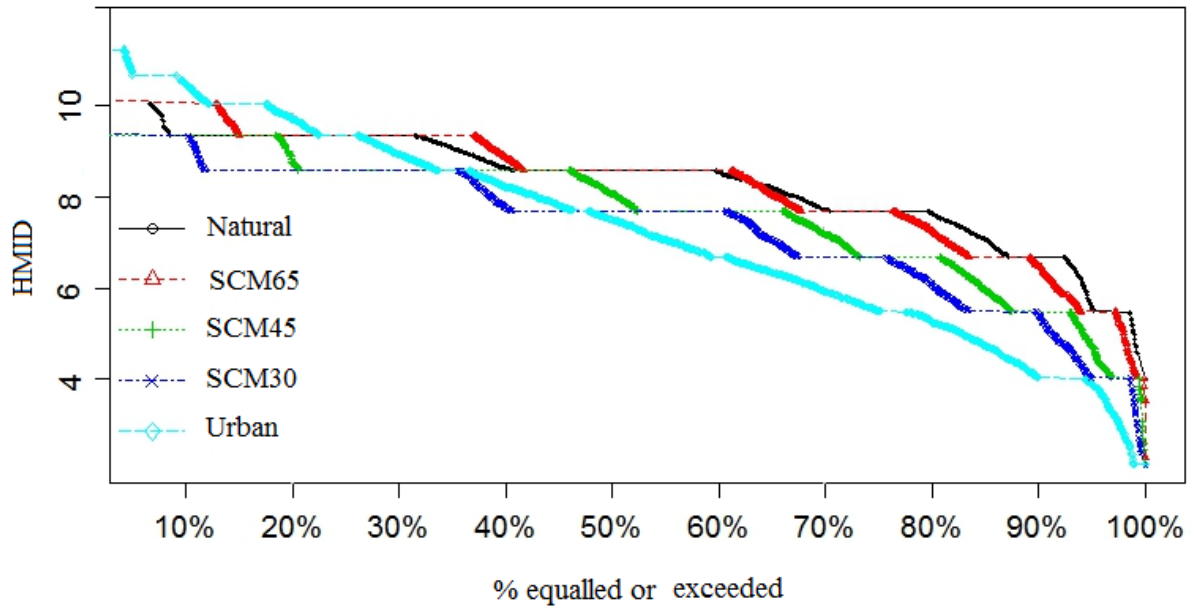


887 Fig. 5. Time series of the daily proportion of the wetted channel bed area under each
888 classification of sediment mobility, for each modelled scenario considered over the study
889 period.



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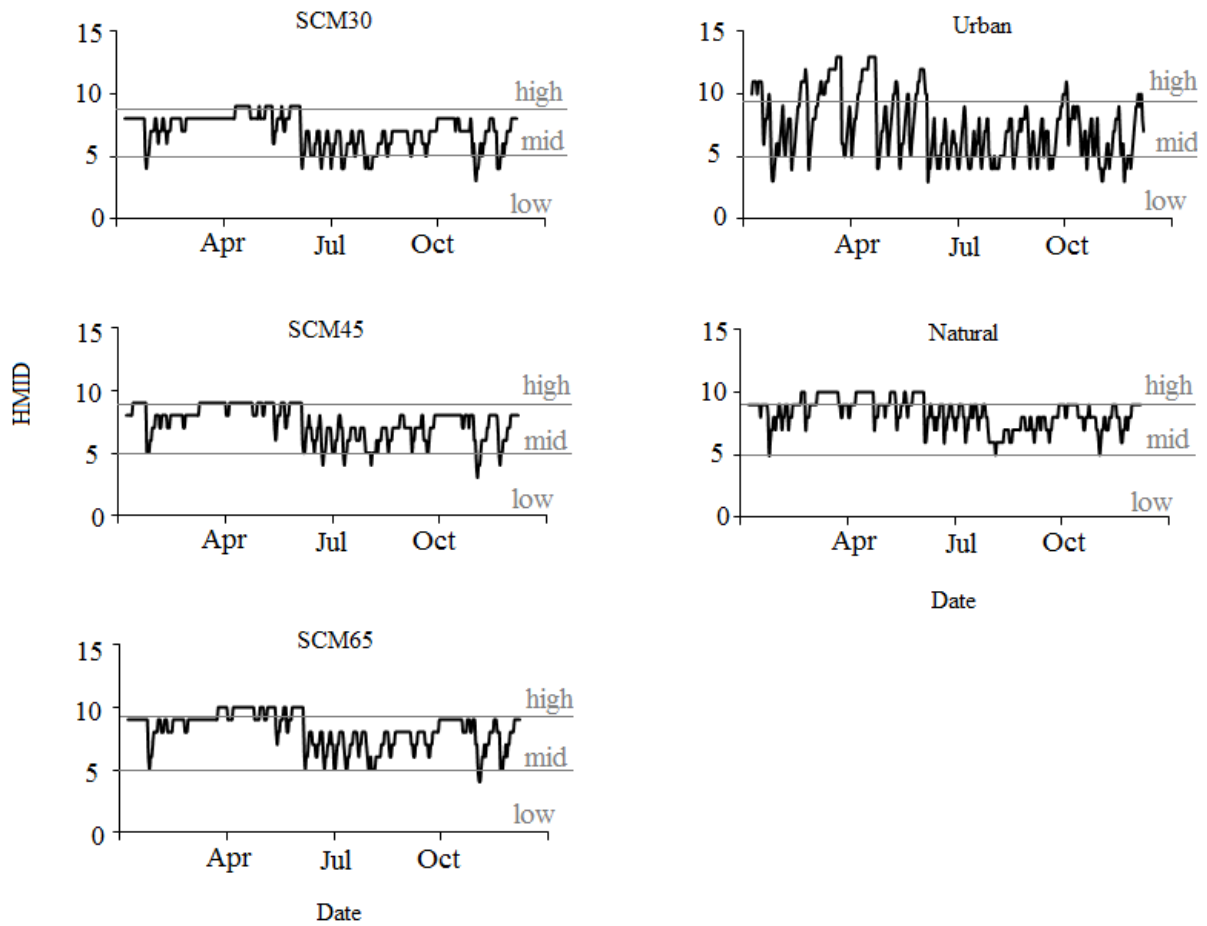
891 Fig. 6. (a) Frequency (in days) that at least 50% of the wetted channel bed area exhibiting
 892 at least partial bed mobility (i.e. severe disturbance); (b) continuous duration of severe
 893 disturbance of each modelled scenario considered over the study period.



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895 Fig. 7. Hydromorphic index of diversity (HMID) percent exceedance curves for each modelled
896 scenario considered over the study period.

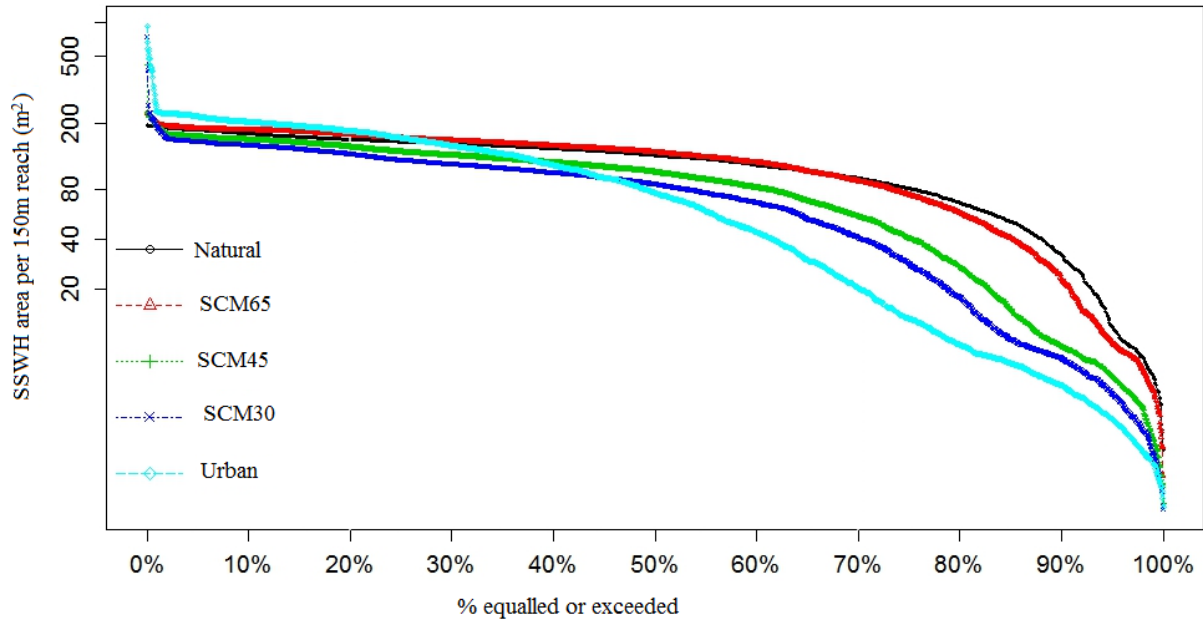
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898 Fig. 8. Time series of daily HMID values for each modelled scenario showing periods of low,

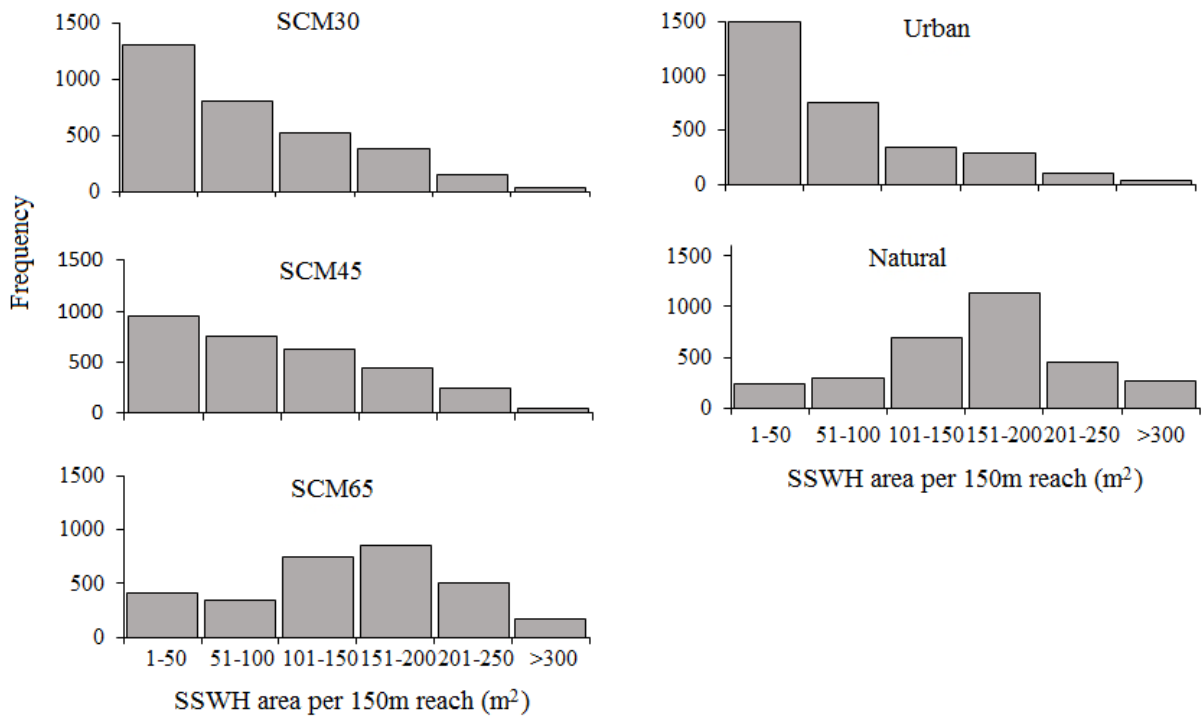
899 medium (mid) and high hydraulic diversity over a year.



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901 Fig. 9. SSWH area percent exceedance curves for each modelled scenario considered over the

902 study period.



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904 Fig. 10. Distribution of daily values of SSWH area for each hydrologic scenario considered

905 over the study period.

906