Thermo-hydraulic Responses of Unsaturated Sand Around a Model Energy Pile

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29 Abstract

30 This paper examines the effects of monotonic and cyclic temperature changes of a 31 model energy pile (diameter = 25 mm and length = 264 mm) on the variations in temperature 32 and volumetric water content of surrounding unsaturated sand. Water flowed away from the pile during heating to 36°C and towards the pile during cooling to 5°C, causing soil drying and 33 34 wetting near the pile, respectively. The change in volumetric water content was time-35 dependent, non-linear and slower than the change in soil temperature and continued to evolve after the soil temperature changes stabilized. Cyclic heating/cooling induced lower thermo-36 37 hydraulic changes in the soil than monotonic heating and cooling. The most significant changes 38 in soil temperatures and volumetric water content were closest to the pile at a radial distance of 20 mm from the edge of the pile and reduced with increasing radial distance for all cases. 39 40 The largest change in the degree of saturation was near the pile and was up to 6% for monotonic 41 heating. Cyclic heating/cooling induced irreversible cyclic hydraulic responses near the pile 42 with consecutive thermal cycles and caused a permanent reduction in the soil volumetric water 43 content. However, these irreversible cyclic effects were dominant at a radius of 20 mm and reduced with increasing radial distance from the energy pile. The change in volumetric water 44 45 content was time-dependent, indicating that the ratio of heating to cooling times during cyclic 46 heating/cooling will have a significant effect on the reversibility of hydraulic responses under temperature cycles. 47

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- 49 *Keywords*: Energy pile; temperature cycles; unsaturated soil; thermo-hydraulic response.
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54 Introduction

The temperatures of energy piles and surrounding soils vary according to the heating 55 and cooling cycles of ground source heat pumps (GSHPs). Any changes in the soil's 56 57 temperatures caused by the cyclic or monotonic operation of the GSHPs will induce water movement within the soil's pores, particularly for unsaturated soils which are multi-phase 58 59 porous media. Thermally-induced water flow in unsaturated soils occurs due to the influence of temperature on the changes in properties of the pore water (e.g. density, viscosity, surface 60 tension) and due to evaporation and condensation of pore water (Philip and DeVries 1957; 61 62 Smits et al. 2013; Baser et al. 2018). The thermo-hydraulic variations of unsaturated soils could potentially affect the piles' thermal and geotechnical performance. 63

The thermal performance of the pile is affected due to an increase or reduction in the 64 soil thermal conductivity when the soil gains or loses water, respectively, which affects the 65 66 heat transfer between the pile and the ground (e.g. Akrouch et al. 2016; Coccia and McCartney 67 2016; Wang et al. 2016; Başer et al. 2018; Hedayati-Dezfooli and Leong 2019; Sani and Singh 68 2020). The mechanism of heat transfer between the energy pile and the soil (very often unsaturated) is, therefore, a combination of conduction and convection rather than the 69 70 commonly assumed conduction being the primary mechanism (Wang and Qi 2011; Moradi et 71 al. 2015; Moradi et al. 2016; Başer et al. 2018). Thomas and Rees (2009), Choi et al. (2011) 72 and Akrouch et al. (2016) indicated that the thermal efficiency of energy piles tended to reduce 73 when the soil degree of saturation reduces due to drying.

Temperature variations of unsaturated soils affect their thermo-hydro-mechanical behaviour (Uchaipichat and Khalili 2009; McCartney et al. 2014; Coccia and McCartney 2016). Changes in the soil thermo-hydro-mechanical behaviour would also affect the coupling between the energy pile and the soil, hence the pile's long-term geotechnical performance. Wang et al. (2015) investigated the shaft capacity of a field-scale energy pile installed in

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79 unsaturated sand using Osterberg cells for static load testing. They found that the pile shaft capacity increased after heating. An early centrifuge modelling study by McCartney and 80 81 Rosenberg (2011) also observed an increase in the ultimate axial capacity of semi-floating 82 energy piles in unsaturated silt with increasing temperature and hypothesised that this was due to an increase in radial stresses due to differential expansion of the pile and soil. However, later 83 84 centrifuge modelling studies on semi-floating energy piles in unsaturated silt and dry sand by 85 Goode and McCartney (2015) found an increase in ultimate axial capacity was only observed 86 in unsaturated silt. They hypothesised that the increases in axial capacity of energy piles with 87 increasing temperature were caused by an increase in soil effective stress due to drying triggered by the heating process. Behbehani and McCartney (2020) recently analysed 88 unreported dielectric sensor measurements from Goode and McCartney (2015). They found 89 90 that thermally-induced drying was the cause of the observed increase in axial capacity, 91 confirming the importance of considering the impacts of coupled heat transfer and water flow 92 on energy pile behaviour.

93 Several physical and numerical studies have generally shown that drying occurs in unsaturated soils in the vicinity close to an underground heat source during heating (e.g. 94 Thomas et al. 2001; Wang and Qi 2011; Wang et al. 2012; Goode 2013; Chen et al. 2014; 95 96 Moradi et al. 2015; Wang et al. 2015; Başer et al. 2016; Coccia and McCartney 2016; Chen et 97 al. 2018; Başer et al. 2018; Jahangir et al. 2018; Cherati and Ghasemi-Fare 2019; Hedayati-98 Dezfooli and Leong 2019; Başer and McCartney 2020; Gao et al. 2020). Studies conducted on 99 thermo-hydraulic responses of unsaturated soils during monotonic cooling are scarce. A 100 preliminary study conducted on monotonic cooling of a model energy pile in loose unsaturated 101 sand has shown evidence of an increase in volumetric water content near the pile (Cameron et 102 al. 2016).

103 Cyclic temperature variations result from seasonal or daily intermittent operations of 104 the GSHP with either natural or forced ground thermal recoveries (Brandl 2006; Yi et al. 2008; 105 Wood et al. 2010; Dai et al. 2015; Faizal et al. 2016; Murphy and McCartney 2015; McCartney 106 and Murphy 2017; Faizal et al. 2018, 2019a, 2019b). Cyclic temperature changes, particularly 107 for systems with daily forced recharging using solar energy or cooling towers, induce frequent 108 temperature reversals of the pile and the soil (Faizal et al. 2016; Faizal and Bouazza 2018). 109 This would ideally cause frequent hydraulic reversals, and hence, repetitive drying/wetting 110 cycles of the soil which could lead to a complex response of stresses in the soil. The frequent 111 cyclic thermo-hydraulic responses of unsaturated soils around energy piles are not well 112 understood yet. A few physical model studies on cyclic temperature changes of energy piles in 113 unsaturated soils have shown that the changes in the degree of saturation in one thermal cycle 114 tend to recover in the following cycle, but the recovery of the degree of saturation is much 115 lower than the restoration of soil temperatures (Moya et al. 1999; Goode 2013; Stewart and 116 McCartney 2014; Cameron et al. 2016). This was confirmed through simulations of coupled 117 heat transfer and water flow surrounding a geothermal heat exchanger in an unsaturated soil 118 layer by Baser et al. (2018), albeit for relatively high temperatures representative of a heat 119 storage system. Therefore, cyclic heating/cooling has the potential to cause irreversible changes 120 in soil water content and permanent drying or wetting of the soil near the pile. Further studies 121 are required to investigate cyclic thermo-hydraulic responses of unsaturated soils around 122 energy piles under cyclic temperature changes.

An assessment of the thermally induced hydraulic responses of unsaturated soils around energy piles under typical monotonic or cyclic temperatures is needed to improve our understanding of this complex coupled heat and mass transfer problem. This paper sheds some light on the above issue; particularly, it examines the thermo-hydraulic responses of an unsaturated soil layer surrounding a model energy pile when subjected to temperature changes 128 typically encountered in practice. The variations in soil temperatures and volumetric water 129 content are physically assessed at different radial distances from the edge of the pile for 130 monotonic heating, monotonic cooling, and cyclic heating/cooling of the energy pile. Although 131 the laboratory-scale tests in this study are not intended to represent a field-scale energy pile, 132 the heat transfer and water flow processes occurring in the laboratory-scale test are the same 133 as those occurring at the field scale, so this study helps understand the range of transient 134 processes that may occur in unsaturated soils close to energy piles. This is particularly important as the soil closest to the energy pile may have the greatest effect on the pile's 135 mechanical response. 136

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138 Experimental Setup and Procedure

139 The experiments were conducted on a concrete model energy pile embedded in 140 compacted, unsaturated sand. A schematic of the experimental setup is shown in Figure 1. The 141 diameter and length of the energy pile were 25 mm and 264 mm, respectively. The concrete 142 used consisted of 4.5 mm aggregates and water-to-cement ratio of 0.42. The average uniaxial compressive strength of unreinforced cylindrical concrete samples was 45.4 MPa after 35 days 143 of curing. The concrete's thermal conductivity was 2.2 W/mK and was measured using a 144 145 divided bar apparatus (Barry-Macaulay et al. 2013; Ali et al. 2016). The model pile was not 146 reinforced since this study focused on the soil's thermo-hydraulic response only and not on the 147 thermo-mechanical performance of the pile. The thermo-hydraulic variations of the sand depend on the heat transfer between the energy pile and the sand. A single U-loop heat 148 149 exchanger made of copper tubing with an outer diameter of 4 mm and a thickness of 0.5 mm 150 was cast in the concrete. The sand was compacted in a container made of 15 mm-thick Perspex 151 and had dimensions of 560 mm (length) x 560 mm (width) x 300 mm (height). The top, bottom 152 and sides of the container were insulated with earthen wool with a thermal resistance of 2 m²K/W to prevent the sand from interacting with local environmental conditions (Figure 1c
and Figure 1d).

The sand was compacted to a dry density of 1300 kg/m³ to allow an easier installation 155 156 of the sensors. The soil-water retention curve (SWRC) and the sand's grain size distribution are 157 shown in Figure 2. The SWRC was estimated using the grain size distribution, the sand specific 158 gravity of 2.65, and a porosity of 0.51 (e.g. Aubertin et al. 2003). The sand used had a 159 uniformity coefficient of 2.4 and a gradation coefficient of 1.0 and was classified as poorly 160 graded. The sand was hand-mixed at a target initial gravimetric moisture content of 5% and a 161 total mass of 77.05 kg of wet soil. Losses of water due to evaporation during mixing resulted 162 in an average initial gravimetric water content of 4.4% (corresponding to an initial degree of 163 saturation, S_r , of approximately 11.2%) for the monotonic cooling experiment and 4.7% (corresponding to $S_r = 12\%$) for the monotonic heating and cyclic heating/cooling experiments. 164 165 The initial temperature and volumetric water content of the compacted sand, recorded from the 166 5TM sensors, were approximately 21°C and 0.08 m³/m³, respectively. The compacted sand's 167 thermal conductivity was 1.0 W/mK and was measured using the KD2 Pro thermal needle 168 probe (Barry-Macaulay et al. 2013).

169 The toe of the energy pile was placed on the container's base. The moist sand was 170 compacted around the energy pile in four 45 mm layers using a hand-held mechanical 171 compactor. The embedded depth of the energy pile was thus 180 mm. Each layer of sand was 172 hand-spread evenly in the box and compacted to the desired 45 mm thickness, which was 173 marked on the container's internal walls, ensuring that all the four layers were compacted to 174 the same thickness and density. Each soil layer's gravimetric water content was verified before 175 compaction to ensure consistency between all the layers. The upper surface of each layer of 176 compacted sand was roughened using sandpaper before compacting the next layer to improve

177 contact between the layers. A plastic sheet was placed on the fourth sand layer to prevent direct178 interaction between the sand and the insulation and minimise moisture losses from the sand.

179 Four 5TM dielectric sensors (Decagon Inc. of Pullman, WA) were installed 180 approximately at mid-height of the soil column to monitor volumetric water content (VWC) 181 and temperatures simultaneously at radial distances, R, of 20 mm, 50 mm, 80 mm, and 110 mm 182 from the edge of the pile (i.e. $R = R_{sensor} - R_{pile}$ from the center of the pile). The sensors were 183 located on planes 90° from each other to minimize interference on the heat transfer and water flow process during the experiments (Figure 1b). The sensors were pushed vertically in the 184 185 sand after compacting the third sand layer, followed by the fourth layer's compaction to prevent 186 the influence of room environmental conditions on the sensors' responses during experiments. 187 A Decagon EM50 data logger recorded the VWC and temperature continuously from the 188 sensors at 2-minute intervals.

189 The VWC sensors were calibrated against different gravimetric water contents of the sand (i.e. approximately 3%, 5%, 7%, and 9%) at a dry density of 1300 kg/m³. The calibration 190 191 slopes of the four sensors are shown in Figure 3. The moist sand was compacted in a Polyvinyl 192 chloride cylindrical container with an external diameter of 250 mm, a wall thickness of 7 mm 193 and a height of 255 mm. The sand was hand spread evenly in the container and compacted in 194 three 45 mm layers using a hand-held mechanical compactor. The contact between the 195 compacted layers was improved by roughening each layer's upper surface with sandpaper. The 196 sensors were pushed vertically in the sand after compacting the third layer, and the soil 197 temperature and the VWC were recorded using the Decagon EM50 data logger. The average temperature of the compacted sand was approximately 22°C. Similar to the study of Moradi et 198 199 al. (2016), it was assumed that temperature did not affect the inferred volumetric water content 200 from the dielectric sensors.

201 Two water baths (model: LAUDA ECO RE 620) were used to circulate warm and cold 202 water in the U-loop of the energy pile at set-point temperatures, simulating monotonic heating 203 and cooling, respectively. The cyclic heating/cooling experiment was conducted by heating the 204 energy pile for 24 hours, followed by cooling for 24 hours. The warm and cold water was 205 circulated at approximately 0.6 liters/min through the heat exchange tubing, which 206 corresponded to a Reynolds number of 2957 during cooling to 5°C and 6070 during heating to 207 36°C. These correspond to turbulent flow conditions in the heat exchanger, which is desirable 208 for optimal heat exchange. The inlet and outlet water temperatures at the pile head were recorded using two thermocouples. Two thermocouples were placed on the container's inner 209 210 edges to monitor boundary thermal effects. The data from the thermocouples were continuously 211 logged using a Pico Technology's USB-TC08 data logger at 5-minute intervals. The heating, 212 cooling, and cyclic heating/cooling experiments were conducted independently of each other 213 by repeating the experimental procedure for each test.

214 Although the experiments were conducted in a temperature-controlled room, there were instances where the temperature was not constant due to technical issues with the room 215 216 temperature controller. An XC0424 USB Temperature and Humidity Datalogger was used to 217 record the room temperature and humidity during all the experiments. The room air 218 temperatures and the humidity during the experiments are shown in Figure 4. The room 219 temperatures generally remained around 20° C for all experiments, apart from the days where 220 the room temperature controller was unstable. Variations in room humidity were not expected 221 to affect the experimental results since the setup was insulated and the top of the soil column 222 in the container was covered with a plastic sheet.

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224 Results and Discussions

225 The inlet/outlet water and boundary temperatures are shown in Figure 5. The inlet water temperatures were 5°C during cooling (Figure 5a) and 36°C during heating (Figure 5b) 226 227 and cycled between 5°C and 36°C during the cyclic heating/cooling test (Figure 5c). The outlet 228 water temperatures were approximately 1.5°C higher and lower than inlet temperatures during 229 cooling and heating, respectively. The average soil temperatures at the container boundary 230 changed by 6°C during monotonic cooling (Figure 5a) and by 3°C during monotonic heating 231 and cyclic heating/cooling (Figure 5b and Figure 5c, respectively). However, these boundary 232 temperature changes remained constant for the duration of the experiments. Hence any 233 boundary effects on the results were also constant. The difference in room temperatures slightly 234 affected the boundary soil temperatures on Day 4 of the cooling test and Day 9 of the heating 235 test. There were instances of some data logging issues during the experiments; hence the 236 temperature monitoring on Day 4 of the cyclic experiments was affected for a few hours (Figure 237 5c). The transient temperature changes of the sand, ΔT_{Soil} , the change in volumetric water 238 content of the sand, $\Delta\theta$, and the change in the sand degree of saturation, ΔSr , at different radial 239 distances from the edge of the model energy pile are shown in Figure 6. The soil temperatures at Day 4 and Day 9 increased by around 1°C for monotonic cooling (Figure 6a) and monotonic 240 241 heating (Figure 6d), respectively, due to an increase in room temperatures on these two days. 242 The soil temperatures closely followed the trends of the inlet fluid temperatures and reached 243 thermal equilibrium with operating time and remained stable for all experiments. The cyclic 244 temperature changes returned to similar peak values at the end of heating and cooling for a 245 given radial distance (Figure 6g). Cyclic heating/cooling induced slightly lower soil 246 temperature changes than monotonic heating and monotonic cooling due to the frequent 247 temperature reversals of the energy pile. The largest ΔT_{Soil} for all experiments was close to the 248 pile and reduced with increasing radial distance from the pile's edge.

249 Monotonic cooling reduced the ground temperature below the initial conditions 250 temperature and induced negative soil thermal gradients (Figure 6a). Thus, water moved 251 towards the pile during monotonic cooling, causing an increase in $\Delta\theta$ and ΔSr due to wetting 252 of the soil (Figure 6b and Figure 6c). On the other hand, monotonic heating induced positive 253 temperature changes (Figure 6d). Thus, water moved away from the pile and $\Delta\theta$, and ΔSr 254 reduced due to soil drying during monotonic heating (Figure 6e and Figure 6f). The cyclic 255 heating/cooling induced a repetitive drying and wetting process during heating and cooling 256 cycles, respectively, where $\Delta\theta$ and ΔSr shows a cyclic response due to the cyclic nature of soil temperature changes (Figure 6g, Figure 6h and Figure 6i). 257

258 Unlike ΔT_{Soil} , $\Delta \theta$ and ΔSr varies with operating time for a given experiment and is not 259 stable, i.e. the VWC does not directly follow the trends of inlet fluid temperatures and continues 260 to change when the soil temperatures had stabilized. There is also a time lag in $\Delta\theta$ and ΔSr 261 between different radial distances with the soil closest to the pile at R = 20 mm undergoing an 262 earlier change in volumetric water content, followed by R = 50 mm, R = 80 mm and R = 110263 mm. It is important to note that the VWC changed eventually with time due to the sustainable 264 application of a constant ΔT_{Soil} , even though the change in VWC was slower than soil 265 temperature change. This indicates that the moisture movement at any given radial location 266 depends on the magnitude of the soil temperature change and the duration that temperature change is maintained; longer time with a fixed ΔT_{Soil} will eventually lead to water movement 267 268 in unsaturated soils.

The largest magnitudes of $\Delta\theta$ and ΔSr are closer to the pile at R = 20 mm (i.e. closer to the heat source) for all experiments and reduces with increasing radial distance from the edge of the energy pile. The amplitude of $\Delta\theta$ and ΔSr for cyclic heating/cooling also decreases with increasing radial distance from the heat source (Figure 6h and Figure 6i). The $\Delta\theta$ and ΔSr at R = 20 mm for cyclic heating/cooling (Figure 6h and Figure 6i) undergoes a progressive 274 reduction with time since heating was the first cycle and the subsequent cooling time was 275 insufficient to return $\Delta\theta$ and ΔSr to initial conditions, even though ΔT_{Soil} was reversed beyond 276 initial conditions temperatures (Figure 6g). This confirms that the hydraulic response is much 277 slower than temperature response and that the ratio of heating to cooling times has a significant 278 effect on the water movement. Compared to the initial degree of saturation, the ΔSr increased 279 by approximately 5% for monotonic cooling (Figure 6c), decreased by about 6% for monotonic 280 heating (Figure 6f) and decreased by around 4% for cyclic heating/cooling (Figure 6i) at R =281 20 mm. There are slight differences in $\Delta\theta$ and ΔSr between monotonic heating and monotonic cooling, particularly at R = 80 mm and R = 100 mm. This could have occurred due to the 282 283 differences in temperature-induced properties of the water in the pores, which could have 284 affected convection in the sand. For example, heating could have increased convection due to 285 a larger influence on water density and viscosity compared to cooling. However, further studies 286 on flow visualisation using numerical studies are required to explain this observation.

287 A comparison of the effect of cyclic heating/cooling against monotonic cooling and monotonic heating at a given radial distance is shown in Figure 7. As discussed earlier, heating 288 289 and cooling induce moisture movement in opposite directions to each other, and $\Delta\theta$ reduces 290 with increasing radial distance. Cyclic heating/cooling causes lower magnitudes of $\Delta\theta$ than 291 monotonic temperature changes. The $\Delta \theta$ during cyclic heating/cooling after R = 50 mm are 292 closer to zero, indicating that variations in the VWC remain near initial conditions compared 293 to monotonic heating and monotonic cooling. Therefore, cyclic temperature operations of 294 GSHPs would be useful for long-term operation of energy piles due to lower impacts on the 295 thermo-hydraulic conditions of the surrounding soil. Long-term physical tests with different 296 heating to cooling ratios are needed to confirm this observation since variations in $\Delta\theta$ depend 297 on the period the change in soil temperatures is maintained.

298 It can be also be observed from Figure 7 that the VWC changed earlier at R = 20 mm while there was a time lag at other radial distances. An initial increase in $\Delta\theta$ is observed at 299 R = 80 mm and R = 110 mm before reducing during monotonic heating (Figure 7f and 300 301 Figure 7h). Similarly, there is an initial reduction in $\Delta \theta$ before increasing during monotonic 302 cooling at R = 80 mm and R = 110 mm. This could be attributed to mass balance as water 303 moves between different radial distances in unsaturated soils under temperature gradients 304 (Chen et al. 2014; Chen et al. 2018; Cherati and Ghasemi-Fare 2019). For example, as VWC reduces and moves away from near the pile during heating, there is an initial increase in VWC 305 at other radial distances. This initial increase in VWC is why there is a time lag in VWC 306 307 reduction during heating at farther radial distances. Similarly, opposite movement of water and 308 mass balance occurs during cooling as water moves from farther radial distances towards the 309 energy pile. This hydraulic balance between different radial distances could also be why $\Delta \theta$ is 310 slightly lower at R = 80 mm than R = 110 mm during cyclic heating/cooling.

311 The thermal and hydraulic radial thermal influence zones are further assessed in 312 Figure 8 by plotting ΔT_{Soil} and $\Delta \theta$ against different radial distances and at different days of operation. Average magnitudes of ΔT_{Soil} and $\Delta \theta$ are shown for cyclic heating/cooling. For a 313 314 given experiment, the ΔT_{Soil} magnitudes were similar for all the days at a given radial distance 315 (Figure 8a) since the temperatures had stabilized with operating time. The ΔT_{Soil} magnitudes 316 reduced with increasing radial distance for monotonic heating and monotonic cooling, while 317 the average soil temperature changes remained almost constant and close to zero for cyclic heating/cooling. The $\Delta\theta$ magnitudes shown in Figure 8b also reduce with increasing radial 318 319 distance from the energy pile and has a similar radial influence zone as ΔT_{Soil} . However, the $\Delta \theta$ 320 magnitudes for a given experiment increase with the increasing number of days for any given 321 radial distance, and as discussed earlier, occur due to the prolonged time ΔT_{Soil} is maintained.

322 The $\Delta\theta$ magnitudes versus ΔT_{Soil} are plotted in Figure 9 to better assess the temperature-323 dependent response of $\Delta \theta$. For monotonic heating and monotonic cooling, $\Delta \theta$ reduced and increased, respectively, approximately between $\Delta T_{Soil} = 6^{\circ}$ C to 11°C at all radial distances, 324 325 confirming that $\Delta\theta$ can increase/decrease at a constant ΔT_{Soil} and that $\Delta\theta$ does not have a linear 326 relationship with ΔT_{Soil} . The $\Delta \theta$ magnitudes during cyclic heating/cooling showed different 327 responses at different radial distances. The $\Delta\theta$ magnitudes at R = 20 mm showed a cyclic 328 hysteresis with irreversible responses against a stable range of ΔT_{Soil} (Figure 9a), indicating 329 that permanent reduction in volumetric water content occurred. As discussed for Figure 6, 330 heating was the first cycle and the subsequent cooling time was insufficient to return $\Delta\theta$ to 331 initial conditions at R = 20 mm in the cyclic heating/cooling test, hence $\Delta \theta$ reduced with 332 consecutive thermal cycles leading to an irreversible response against ΔT_{Soil} . It is also likely 333 that soil drying during heating could have reduced the hydraulic conductivity of the soil which 334 prevented a return of water during cooling (Coccia and McCartney 2016; Baser et al. 2018); 335 this, however, cannot be confirmed from the current results and requires further physical and 336 numerical studies. The $\Delta\theta$ magnitudes during cyclic heating/cooling at other radial distances 337 (Figure 9b, Figure 9c, and Figure 9d) showed insignificant changes with consecutive thermal 338 cycles, indicating that the zone of influence of permanent increase/decrease of $\Delta \theta$ is closer to 339 the pile at R = 20 mm and reduces with increasing radial distance.

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341 Conclusions

This paper examined the variations in temperatures and volumetric water content of unsaturated sand around a model energy pile (diameter = 25 mm and length = 264 mm) when the pile was subjected to monotonic heating, monotonic cooling, and frequent cyclic heating/cooling. Water moved away from the pile during heating and towards the pile during cooling, causing drying and wetting of the soil, respectively. Cyclic responses of change in the

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347 volumetric water content were observed near the pile during cyclic heating/cooling. The change 348 in volumetric water content was non-linear and slower with respect to soil temperature changes. 349 This resulted in permanent drying of the soil closest to the energy pile after several cycles of 350 heating and cooling. Cyclic heating/cooling induced lower changes in soil temperatures and 351 volumetric water content than monotonic heating/cooling and would, therefore, be useful for 352 long-term energy piles operations. The largest changes in soil temperatures and volumetric 353 water content were closer to the energy pile at a radial distance of 20 mm and reduced with 354 increasing radial distance for all cases. The change in volumetric water content did not stabilize with operating time. It continued to evolve even though soil temperatures had stabilized, 355 356 indicating that soil moisture variations depend on the magnitudes of change in soil temperatures and the duration the temperature change is maintained. The zone of radial thermal influence of 357 temperature changes and change in volumetric water content was similar. 358

359 The change in volumetric water content near the pile reduced with consecutive thermal 360 cycles. It showed irreversible responses against change in soil temperatures during cyclic 361 heating/cooling, indicating a permanent reduction in volumetric water content near the pile at a radius of 20 mm. However, these irreversible cyclic effects reduced with increasing radial 362 363 distance from the pile. Since water movement depends on the magnitude and duration of soil 364 temperature change, the ratio of heating to cooling times during cyclic heating/cooling significantly affects the reversibility of hydraulic responses under temperature cycles. This 365 366 paper's results and conclusions are based on a small-scale model energy pile embedded in an 367 idealised sand layer and controlled boundary conditions. Even though the results present useful 368 insights into sand's thermo-hydraulic behaviour around energy piles, further in-situ tests are 369 still warranted to understand the temperature dependant water movement in the soil under real 370 boundary conditions. Finally, the results presented in this paper are for a single energy pile.

371	Further studies ar	re required to asses	s the thermo-hydraulio	c responses of o	closely spaced	energy
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372 piles operating in groups where soil temperature changes are expected to be higher.

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374 **Data Availability Statement**

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All data, models, and code generated or used during the study appear in the submitted 376 article.

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Figure 1. Experimental setup : (a) cross-sectional elevation view; (b) cross-sectional plan view; (c) partially insulated; and (d) fully insulated.



Figure 2. (a) Soil water retention curve; and (b) particle size distribution.



Figure 3. Volumetric water content inferred from the dielectric sensors (VWC_{measured}) used at the four radial locations, R, in the experimental setup versus the volumetric water content obtained from oven drying (VWC_{corrected}).



Figure 4. Room temperature and humidity during experiments: (a) cooling test; (b) heating test; and (c) cyclic heating/cooling test.



Figure 5. Fluid (water) and boundary temperatures: (a) cooling test; (b) heating test; and (c) cyclic heating/cooling test.



Figure 6. Change in soil temperatures (ΔT_{Soil}), change in soil volumetric water content ($\Delta \theta$), and change in degree of saturation (ΔSr), at different radial locations: (a), (b) and (c) ΔT_{Soil} , $\Delta \theta$, and ΔSr for cooling test, respectively; (d), (e) and (f) ΔT_{Soil} , $\Delta \theta$, and ΔSr for heating test, respectively; and (g), (h) and (i) ΔT_{Soil} , $\Delta \theta$, and ΔSr for cyclic heating/cooling test, respectively.



Figure 7. Comparison of ΔT_{Soil} and $\Delta \theta$ for monotonic and cyclic temperature changes of the soil at given radial distances, *R*: (a) and (b) ΔT_{Soil} and $\Delta \theta$ at R = 20 mm, respectively; (c) and (d) ΔT_{Soil} and $\Delta \theta$ at R = 50 mm, respectively; (e) and (f) ΔT_{Soil} and $\Delta \theta$ at R = 80 mm respectively; and (g) and (h) ΔT_{Soil} and $\Delta \theta$ at R = 110 mm, respectively.



Figure 8. Thermal and hydraulic radial influence zones: (a) change in soil temperatures (ΔT_{Soil}) ; and (b) change in soil volumetric water content ($\Delta \theta$).



Figure 9. Change in volumetric water content ($\Delta\theta$) plotted against change in soil temperature (ΔT_{Soil}): (a) R = 20 mm; (b) R = 50 mm; (c) R = 80 mm; and (d) R = 110 mm.